

Traffic Generation Example

The following Traffic Generation Example has been updated and is provided to give a better understanding of the methodology for creating the Traffic Generation Diagram.

Please note that DelDOT's <u>Development Coordination Portal</u> and the <u>Institute of Transportation</u> <u>Engineer's Trip Generation Manual and Handbook</u> provide worksheets and tools for determining various factors necessary to create a Trip Generation Diagram; however, these worksheets and tools are periodically updated and may cause inconsistencies with the examples below. The latest DelDOTapproved editions of the ITE Trip Generation Manual and DelDOT roadway traffic data should be utilized.

Road Traffic Data:

- 1. Functional Classification and Roadway Maintenance Number Information within the <u>DelDOT</u> <u>Gateway</u>
- 2. Posted Speed Actual posted speed from the roadway Google Street View
 - If roadway does not have a posted speed limit, can assume 50 MPH.
 - Speed limit resolution information is available within the <u>DelDOT Gateway</u>, which should be verified with the Google Street View information.
- 3. AADT Information from the latest <u>DelDOT Gateway</u>
- 10-Year Projected AADT = Current ADT x 10-Year Growth Factor. Apply the same 10-Year Growth Factor that is used in DelDOT's <u>Auxiliary Lane Worksheet</u>. Currently the growth factor is 1.16. The most up-to-date worksheet is located within DelDOT's <u>Development Coordination</u> <u>Portal</u>
- 5. Traffic Pattern Group Information from the <u>DelDOT Gateway</u>
- 6. K-Factor, D-Factor, Combo Unit Truck Percentage Information from the latest DelDOT <u>Traffic</u> <u>Volume Summary</u>

Site Traffic Data:

- 1. Trip generation is based on the latest edition of the ITE Trip Generation Manual approved by DelDOT. The procedure to determine if rates and/or fitted curve equations are to be used to calculate the trip generation for the proposed development is based on the guidance provided in the latest ITE Trip Generation Handbook.
 - a. Typically, trip generation should be provided for the weekday ADT (Average Daily Trips), weekday AM peak hour trips, and weekday PM peak hour trips. However, additional ADT and peak hour trips (such as Saturday and Sunday data) may be required to be included on the diagram at DelDOT's discretion. A credit may be taken for pass-by and internal capture trips. For more information on pass-by trips and internal capture trips, please refer to the latest edition of the ITE Trip Generation Manual approved by DelDOT.
- 2. Number of entrances and corresponding configurations
- 3. Design Vehicle Select a design vehicle that is likely to use the proposed development entrances with considerable frequency, ensures that the entrance facility can accommodate the anticipated turning movements, and is consistent with Figure 5.2.3-a from the Development Coordination Manual.
- 4. Existing Site Traffic (if applicable)
 - a. If the proposed site will replace an existing site that has been active within the past 3 years, a credit for the existing site traffic may be applied at DelDOT's discretion.
- 5. Total Site Traffic for Proposed Development
- 6. Net Change in New Traffic between Existing Site and Proposed Development (if applicable)
 - a. May consider internal capture trips, pass-by trips, and existing site trips
 - b. Required to determine if the proposed site meets any Traffic Impact Study (TIS) or Area-Wide Study Fee (AWSF) warrants.
- 7. Any other trips (i.e. from adjacent sites that share the same access) that would utilize the access from external sites (if applicable)
 - a. Required to be depicted to determine the total trips to include in the auxiliary lane worksheet.
 - b. These trips do not count towards the TIS or AWSF volume warrants for the proposed site.
- 8. Directional Distribution: If information specific to the site occupant is available, for example, the feeder pattern for a school site, that information should be used. More commonly, this distribution is based on engineering judgment which is based upon existing and /or future traffic patterns surrounding the development area. Details of determining peak hour volumes are located in the **Traffic Generation Diagram** section below. Development generated trips should be assigned to the proposed entrances according to the actual development site plan as well as existing and/or future traffic patterns surrounding the development area.
- 9. Percent of Peak Hour Heavy Vehicles at Site Select a percentage that is consistent with the proposed development.

Example Project – One Site Entrance:

A residential development is proposed on the easterly side of Autumn Moon Lane south of Walnut Shade Road in Kent County. The development consists of 52 single family detached units and 96 multifamily housing (low-rise). The site proposes one entrance along Autumn Moon Lane. The site does not have any existing buildings and does not propose to share access with adjacent sites.

• Road Traffic Data:

- 1. Functional Classification and Roadway Maintenance Number Information within the <u>DelDOT</u> <u>Gateway:</u> K-369 Autumn Moon Lane – Local
- 2. Posted Speed Actual posted speed from the roadway Google Street View: 50 MPH
 - If roadway does not have a posted speed limit, can assume 50 MPH.
 - Speed limit resolution information is available within the <u>DelDOT Gateway</u>, which should be verified with the Google Street View information.
- 3. AADT Information from the latest <u>DelDOT Gateway:</u> **1,465 Trips** (From 2023 DelDOT Traffic Summary)
- 4. 10-Year Projected AADT = Current ADT x 10-Year Growth Factor. Apply the same 10-Year Growth Factor that is used in DelDOT's <u>Auxiliary Lane Worksheet</u>. Currently the growth factor is 1.16. The most up-to-date worksheet is located within DelDOT's <u>Development Coordination</u> <u>Portal</u>:

10-year AADT = 1,465 x 1.16 = 1,699 Trips

- 5. Traffic Pattern Group Information from the <u>DelDOT Gateway</u>: **TPG 4**
- 6. K-Factor, D-Factor, Truck Percentage Information from the latest DelDOT <u>Traffic Volume</u> <u>Summary</u>:

K-Factor = 13.0, D-Factor = 57.6, Combo Unit Truck % = N/A (From 2023 DelDOT Traffic Summary)

• Site Traffic Data:

- 1. Trip generation is based on the latest edition of the ITE Trip Generation Manual approved by DelDOT to utilize. The guidance provided in the latest ITE Trip Generation Handbook should be used to determine whether to use the average rate and/or the fitted curve equation to calculate the trip generation for the proposed use.
 - i. 52 Single Family Detached Units (ITE Land Use Code 210) Use fitted curve equation which results in 553 ADT (276 Enter/277 Exit), 41 AM Peak (10 Enter/31 Exit), 54 PM Peak (34 Enter/20 Exit)
 - ii. 96 Multifamily Housing (Low-Rise) (ITE Land Use Code 220) Use fitted curve equation which results in 691 ADT (345 Enter/346 Exit), 53 AM Peak (13 Enter/40 Exit), 62 PM Peak (39 Enter/23 Exit)
- 2. Number of entrances and corresponding configurations One Entrance Full Movement
- 3. Design Vehicle SU-30
- 4. Existing Site Traffic None/Vacant
- 5. Total Site Traffic for Proposed Development 1,244 ADT (621 Enter/623 Exit), 94 AM Peak (23 Enter/71 Exit), 116 PM Peak (73 Enter/43 Exit)
- 6. Net Change in New Traffic between Existing Site and Proposed Development N/A
- 7. Other Trips Utilizing Site Access None
- 8. Directional Distribution:

In this case, 85% to and from the north and 15% to and from the south.

- 85% to and from the north = 1,244 x 85% = 1,058 ADT (528 Enter/530 Exit), 80 AM Peak (20 Enter/60 Exit), 99 PM Peak (62 Enter/37 Exit)
- 15% to and from the south = 1,244 x 15% = 186 ADT (93 Enter/93 Exit), 14 AM Peak (3 Enter/11 Exit), 17 PM Peak (11 Enter/6 Exit)
- 9. Percent of Peak Hour Heavy Vehicles at Site Peak Hour HV% of Proposed Site = 5%.

• Traffic Generation Diagram:

- 1. Development generated trips should be assigned to the proposed entrances according to the development site plan as well as existing and/or future traffic patterns surrounding the development area.
- 2. For the purpose of this example, the directional distribution is 85% to and from the north and 15% to and from the south.
 - Site ADT entering from the south = Entering ADT for Proposed Development x Percentage of Proposed Development from the south = $621 \times 15\% = 93$.
 - Site ADT entering from the north = Entering ADT for Proposed Development x Percentage of Proposed Development from the north = $621 \times 85\% = 528$.
 - Site ADT exiting to travel south = Exiting ADT for Proposed Development x Percentage of Proposed Development to south = 623 x 15% = 93.
 - Site ADT exiting to travel north = Exiting ADT for Proposed Development x Percentage of Proposed Development to north = 623 x 85% = 530.

TRAFFIC GENERATION - AUTUMN MOON LANE (K369) (FULL MOVEMENT)	
11860 TI BEEN PROPOSED ENTRANCE	ROAD TRAFFIC DATA: FUNCTIONAL CLASSIFICATION - K-369 (AUTUMN MOON LANE) - LOCAL POSTED SPEED LIMIT - 50 mph ROADWAY AADT = 1,465 TRIPS (FROM 2023 DelDOT TRAFFIC SUMMARY) 10 YEAR PROJECTED ROADWAY AADT = 1.16 X 1,465 TRIPS = 1,699 TRIPS TRAFFIC PATTERN GROUP - 4 (FROM 2023 DELDOT TRAFFIC SUMMARY) K-FACTOR = 13.0, D-FACTOR = 57.6, COMBO UNIT TRUCK % = N/A (FROM 2023 DELDOT TRAFFIC SUMMARY)
	SITE TRAFFIC DATA: SOURCE: ITE TRIP GENERATION MANUAL 11th EDITION. 52 SINGLE FAMILY DETACHED UNITS (210) 96 MULTIFAMILY HOUSING (LOW-RISE) (220) ONE ENTRANCE - FULL MOVEMENT DESIGN VEHICLE: SU-30 52 SINGLE FAMILY DETACHED UNITS = 553 ADT (276 ENTER/277 EXIT), 41 AM PK (10 ENTER/31 EXIT), 54 PM PK (34 ENTER/20 EXIT) 96 MULTIFAMILY HOUSING EVEN (210 MISE) UNITS = 601 ADT (345 ENTER/346 EXIT),
AUTUMN MOON LANE (K369)	TOTAL STRE = 1,244 ADT (621 ENTER/623 EXIT), 94 AM PK (23 ENTER/71 EXIT, 116 PM PK (73 ENTER/43 EXIT) DIRECTIONAL DISTRIBUTION
1,699 (ROADWAY ADT)	85% TO AND FROM THE NORTH 15% TO AND FROM THE SOUTH
TRAFFIC GENERATION DIAGRAM ADT (A.M. PEAK HOUR) [P.M. PEAK HOUR] {DISTRIBUTION}	PEAK HOUK HV% OF PROPOSED SITE = 3%

The same procedure should be applied for the AM and PM peak hour site traffic.

DelDOT Auxiliary Lane Worksheet

Complete the worksheet per below:

- 1. Fill in the volumes on the **Traffic Generation Diagram** tab consistent with the Traffic Generation Diagram created for the Entrance Plan.
 - i. If the proposed entrance will create the fourth leg to an existing entrance, separate Auxiliary Lane Worksheets shall be completed and submitted for review of both the proposed entrance and the existing entrance.
 - ii. If the proposed entrance will create the fourth leg to an existing T intersection (2 roads), an Auxiliary Lane Worksheet shall be completed for the proposed entrance and all other approaches will need further evaluation per the Development Coordination Manual and associated state and federal manuals (i.e. AASHTO Green Book).
- 2. If the entrance is an existing access point, left turn and right turn ADT and peak hour volumes shall include site traffic and existing roadway traffic.
- 3. If opposing roadway traffic volumes were collected, include in the worksheet.
- 4. If the opposing right-turn movement is channelized, a reduction may be included in the worksheet. Justification for reduction shall be submitted to DelDOT Development Coordination Section for review.
- 5. If Committed Development traffic information is known, include in the worksheet. This information is commonly included in a Final TIS.
- 6. In the User Inputs tab, fill in the cells with green text.
 - i. If opposing roadway traffic volumes were collected and/or the opposing right- turn movement is channelized, the Left Turn VPH should be the same peak hour as the peak hour chosen for the opposing through and right turn volumes within the **Traffic Generation Diagram** tab.
- 7. To obtain further clarification on how ADT is determined per approach, see Left-Right Approach Example tab within the DelDOT Auxiliary Lane Worksheet.

Creation of DelDOT Auxiliary Lane Worksheet for Proposed Entrance



Roadway AADT From DelDOT Traffic Summary:	1465
Committed Development AADT:	0

May 28, 2024					V2024.1
DeIDOT	Manually Update Cell XX Auto-Calculated Cells XX				
Name of Project	Example - 1 Access	Date of Submittal	6/17/2024	Pro	Pro
Maintenance Road No. (i.e. K234A)	K369	Road Name	Autumn Moon Lane	pose	pose
Signalized / Unsignalized	Unsignalized	Posted Speed Limit	50		1 <u> </u>
Roadway ADT (From DelDOT Traffic Manual)	1465	Traffic Pattern Group	4	*L1	trance
Left Committed Approach 186 Development 0 Site ADT ADT	Total Left Approach 186 ADT	Right Committed Approach 1058 Site ADT ADT	Total Right Approach 1058 ADT	1	
Total Number of Through Lanes (Does Not Include Turn Lanes)	2 lanes	Number of intersection legs	3	taper V	× × · · · ·
Roadway Functional Classification	Local	Calculation for (specify leg)	Proposed Entrance 1		
Left-Approach Projected 10 yr Roadway ADT + Total Site + Committed Development ADT	1885	Right-Approach Projected 10 yr Roadway ADT + Total Site + Committed Development ADT	2757	× 1	5 <u>9</u>
K Factor	13	D Factor	57.6		
Left Turn Information		Right Turn Information			
Left Turn VPH	11	Right Turn ADT	Over 400		- 1 Ac
Left Turn Approach Grade	0.0%	Right Turn Approach Grade	0.0%		40 290
Heavy Vehicle %	5	Effective Radius of Entrance	R≤50'	12	90
10 Yr Opposing Vol. (Manual Input - Veh/hr)	0				
10 Yr Opposing Volume (Calculated)	127 Veh/hr	Right Turn Length	290 ft	taper	, <u>s</u> ,
10 Yr Opposing Volume (Calculated Vol.)	127 Veh/hr			Å	
		Bypass and Left Turn La required	anes are not		

7 Trip Generation

Revised June 2024

Example Project – Two Site Entrances and Existing Use:

A mixed-use development is proposed on the southerly side of Old Coach Road between Upper Pike Creek Road and Polly Drummond Road in New Castle County. The development consists of 200,000 square feet of retail space (195,000 square feet of retail and a 5,000 square foot drive in bank). The site proposes two entrances on Old Coach Road. Existing conditions of the site include 55 multifamily housing units (low-rise) to be demolished, and the two proposed entrances on Old Coach Road would also include an adjacent site of 100 single-family detached homes.

• Road Traffic Data:

Functional Classification and Roadway Maintenance Number – Information within the <u>DelDOT</u> <u>Gateway:</u> N-316 Old Coach Road – Major Collector

- 1. Posted Speed Actual posted speed from the roadway Google Earth: 35 MPH
- 2. AADT Information from the latest <u>DelDOT Gateway:</u> **5,823 Trips** (From 2023 DelDOT Traffic Summary)
- 10-Year Projected AADT = Current ADT x 10-Year Growth Factor. Apply the same 10-Year Growth Factor that is used in DelDOT's <u>Auxiliary Lane Worksheet</u>. Currently the growth factor is 1.16. The most up-to-date worksheet is located within DelDOT's <u>Development Coordination</u> <u>Portal</u>:

10-year AADT = 5,823 x 1.16 = **6,755 Trips**

- 4. Traffic Pattern Group Information from the <u>DelDOT Gateway</u>: **TPG 2**
- 5. K-Factor, D-Factor, Truck Percentage Information from the latest DelDOT <u>Traffic Volume</u> <u>Summary</u>:

K-Factor = 10.8, D-Factor = 56.2, Combo Unit Truck % = 2.6 (From 2023 DelDOT Traffic Summary)

• Site Traffic Data:

- 1. Trip generation is based on the latest edition of the ITE Trip Generation Manual. The guidance provided in the latest ITE Trip Generation Handbook should be used to determine whether to use the average rate and/or the fitted curve equation to calculate the trip generation for the proposed use. Peak hour periods to include in the diagram is at DelDOT's discretion. In this specific example, only AM peak hour and PM peak hour values are represented.
 - i. 200,000 square feet Strip Retail Plaza (ITE Land Use Code 820) Use fitted curve equation for ADT, AM, and PM, which results in 11,086 ADT (5,543 Enter/5,543 Exit), 252 AM Peak (156 Enter/96 Exit), 930 PM Peak (446 Enter/484 Exit)
 - Pass-by Trips: Per ITE Trip Generation Manual, 11th Edition, the pass-by average rate for LUC 820 (Shopping Center > 150k) in the Weekday PM Peak Period is 29%.
 - a. PM Peak Period: 930 PM Peak (446 Enter/484 Exit) *29% = 269 (129 Enter/140 Exit)
- 2. Number of entrances and corresponding configurations: Two Entrances Full Movement
- Design Vehicle Select a design vehicle that is likely to use the proposed development entrances with considerable frequency to ensure that the entrance facility can accommodate the anticipated turning movements and is consistent with Figure 5.2.3-a from the Development Coordination Manual. WB-40
- 4. Site Traffic of Existing Site 55 multifamily housing units (low-rise) (ITE Land Use Code 220) Use fitted curve equation which results in **428 ADT**, **40 AM Peak**, **44 PM Peak**
- 5. Total Site Traffic for Proposed Development 11,086 ADT (5,543 Enter/5,543 Exit), 252 AM Peak (156 Enter/96 Exit), 930 PM Peak (446 Enter/484 Exit)
 - a. Total New Site Traffic for Proposed Development (Total Site Traffic Pass-by Trips) 11,086 ADT (5,543 Enter/5,543 Exit), 252 AM Peak (156 Enter/96 Exit), 661 PM Peak (317 Enter/344 Exit)
- 6. Net Change in New Traffic between Existing Site and Proposed Development
 - a. ADT: 11,086 ADT Proposed 428 Existing = 10,658
 - b. AM: 252 AM Peak Proposed 40 Existing = **212**
 - c. PM: 661 PM Peak Proposed 44 Existing = 617
- Other Trips (Adjacent Site Traffic) 100 Single-Family Detached Homes (ITE Land Use Code 210) Use fitted curve equation which results in 1,009 ADT (505 Enter/504 Exit), 74 AM Peak (19 Enter/55 Exit), 99 PM Peak (63 Enter/36 Exit)
- 8. Directional Distribution: If information specific to the site occupant is available, for example, the feeder pattern for a school site, that information should be used. More commonly, this distribution is based on engineering judgment which is based upon existing and /or future traffic patterns surrounding the development area. Details of determining peak hour volumes are located in the **Traffic Generation Diagram** section below.

In this case 75% to and from the west and 25% to and from the east.

- 75% to and from the west = (11,086+1,009) x 75% = 9,071 ADT (4,536 Enter/4,535 Exit), 245 AM Peak (132 Enter/113 Exit), 772 PM Peak (382 Enter/390 Exit)
- 25% to and from the east = (11,086+1,009) x 25% = 3,024 ADT (1,512 Enter/1,512 Exit), 81 AM Peak (43 Enter/38 Exit), 257 PM Peak (127 Enter/130 Exit)
- 9. Percent of Peak Hour Heavy Vehicles at Site Select a percentage that is consistent with the proposed development Peak Hour HV% of Proposed Site = 5%.

• Traffic Generation Diagram:

- 1. Development generated trips should be assigned to the proposed entrances according to the development site plan as well as existing and/or future traffic patterns surrounding the development area.
- 2. For the purpose of this example, the directional distribution is 75% to and from the west and 25% to and from the east. In addition, since there are two entrances, the trips coming in and out of the development are assigned 70% for the first entrance passed and 30% for the second entrance passed. Note, the percentages utilized are assumed for this example and different percentages based on specific aspects of a development may be appropriate.

٠	Site ADT En	terii	ng from 1	the west										
	Entrance 1 =	Ent	ering Sit	e ADT x Di	rectional Dis	tribution x	% at 1 st E	Intrance						
	=	=	6,048	X	75%	X	70%	= 3,175						
	Entrance 2 = Entering Site ADT x Directional Distribution x % at 2^{nd} Entrance													
	=	=	6,048	X	75%	X	30%	= 1,361						
•	Site ADT Exi	iting	g to trave	el west										
	Entrance 1 = Exiting Site ADT x Directional Distribution x % at 1 st Entrance													
	=	:	6,048	X	75%	Х	70%	= 3,175						
	Entrance 2 = Exiting Site ADT x Directional Distribution x % at 2^{nd} Entrance													
	=		6,048	X	75%	X	30%	= 1,361						
•	Site ADT En	terii	ng from t	the east										
	Entrance 1 = 1	Ente	ering Site	e ADT x Dir	ectional Dist	ribution x '	% at 1 st E	ntrance						
	=		6,048	X	25%	Х	30%	= 454						
	Entrance 2 = I	Ente	ring Site	e ADT x Dir	ectional Dist	ribution x 9	% at 2 nd E	ntrance						
	=		6,048	X	25%	X	70%	= 1,058						
•	Site ADT Exi	iting	g to trave	el east										
	Entrance 1 =	Exi	iting Site	ADT x Dir	ectional Dist	ribution x 9	% at 1 st Ei	ntrance						
	=	=	6,048	X	25%	X	30%	= 454						
	Entrance 2 =	Exi	ting Site	ADT x Dire	ectional Dist	ribution x %	∕o at 2 nd E	ntrance						
	=	=	6,048	X	25%	X	70%	= 1,058						

The same procedure should be applied for the AM and PM peak hour site traffic.



• DelDOT Auxiliary Lane Worksheet

Complete the worksheet per below:

- 1. Fill in the volumes on the **Traffic Generation Diagram** tab consistent with the Traffic Generation Diagram created for the Entrance Plan.
 - i. If the proposed entrance will create the fourth leg to an existing entrance, separate Auxiliary Lane Worksheets shall be completed and submitted for review of both the proposed entrance and the existing entrance.
 - ii. If the proposed entrance will create the fourth leg to an existing T intersection (2 roads), an Auxiliary Lane Worksheet shall be completed for the proposed entrance and all other approaches will need further evaluation per the Development Coordination Manual and associated state and federal manuals (i.e. AASHTO Green Book).
- 2. If the entrance is an existing access point, left turn and right turn ADT and peak hour volumes shall include site traffic and existing roadway traffic.
- 3. If opposing roadway traffic volumes were collected, include in the worksheet.
- 4. If the opposing right-turn movement is channelized, a reduction may be included in the worksheet. Justification for reduction shall be submitted to DelDOT Development Coordination Section for review.
- 5. If Committed Development traffic information is known, include in the worksheet. This information is commonly included in a Final TIS.
- 6. In the User Inputs tab, fill in the cells with green text.
 - i. If opposing roadway traffic volumes were collected and/or the opposing right- turn movement is channelized, the Left Turn VPH should be the same peak hour as the peak hour chosen for the opposing through and right turn volumes within the **Traffic Generation Diagram** tab.
- 7. To obtain further clarification on how ADT is determined per approach, see Left-Right Approach Example tab within the DelDOT Auxiliary Lane Worksheet.

Creation of DelDOT Auxiliary Lane Worksheet for Proposed Entrance 1

*Committed Development AADT contains the site traffic to and from Proposed Entrance 2



Roadway AADT From DeIDOT Traffic Summary:	5823
Committed Development AADT:	2722

May 20, 2024																			v	2024.
DelDOT Auxiliary Lane Worksheet Roadway Information and Entrance															Manua Auto-(X	x x			
Name of Project		Example	- 2 Access	Date of Sul	Date of Submittal 6/17/2024				/2024					Proj					Pro	
Maintenance Road No. (i.e. K234	A)	N3	16	Road Name	2			Old Coa	ch Road					pose					pose	
Signalized / Unsignalized		Unsigr	nalized	Posted Spe	ed Limit			3	5					dEn		•			dEn	
Roadway ADT (From DelDOT Traffic Manual)		58	23	Traffic Patt	ern Grou	ıp		2	Ę				trance	\uparrow	7			trance		
Left Committed Approach 908 Developme Site ADT ADT	nt 2722	Total Left Approach ADT	3630	Right Approach Site ADT	6350	Committed Development ADT	2722	Total Right Approach ADT	9072	+	100'	-\		4	×		:		1	
Total Number of Through Lanes (Does Not Include Turn Lanes)		2 la	nes	Number of	intersec	tion legs		a	3		taper				××			≥		
Roadway Functional Classification	1	Major C	ollector	Calculation	Calculation for (specify leg) Proposed Entrance 1					20					13	4				
Left-Approach Projected 10 yr Road Total Site + Committed Developme	384	Right-Approach Projected 10 yr Roadway ADT + Total Site + Committed Development ADT						60		* *		<u>u</u>	∛ \∶∥				Ex			
K Factor		10	.8	D Factor 56.2					↓	1		<u> </u>		Ê	- <u> :</u>		1	\mathbf{T}	Ä	
Left Turn Informati	on			Right Turn Information										V	•	•			ple	
Left Turn VPH		3	8	Right Turn ADT Over 400						ſ	\uparrow		•	1			- 2 Ac			
Left Turn Approach Grade		0.0	0%	Right Turn Approach Grade 0.0%					0.0%					190					240 190	cess
Heavy Vehicle %			5	Effective R	adius of E	Intrance		R≤	50'	-				940 91				95		
10 Yr Opposing Vol. (Manual Inp	ıt - Veh/hr)		נ																	
10 Yr Opposing Volume (Calculated) 410 Veh/hr					Length			240	D ft					50'				a la	2	
10 Yr Opposing Volume (Calculated Vol.) 410 Veh/hr																	4	(<u> </u>	
Left Turn Length 220 ft																	A			

Creation of DelDOT Auxiliary Lane Worksheet for Proposed Entrance 2

*Committed Development AADT contains the site traffic to and from Proposed Entrance 1



Roadway AADT From DeIDOT Traffic Summary:	5823
Committed Development AADT:	908

May 20, 20	Pray 20, 2024 V2024.													:UZ4. I								
DelDOT Auxiliary Lane Worksheet Roadway Information and Entrance													Manu Auto-	XX XX								
Name of Project Example - 2 Access						Date of Sub	Date of Submittal 6/17/2024				2024					Pro					Pro	
Maintenance F	Road No. (i	.e. K234A)		N3	16	Road Name				Old Coa	ch Road					pose					opose	
Signalized / Un	nsignalized			Unsign	alized	Posted Spe	ed Limit			3	5					d Ent		۲			d En	
Roadway ADT (From DelDOT	Traffic Ma	nual)		58	23	Traffic Patt	ern Grou	n Group			2					trance	\uparrow	17			trance	
Left Approach 2 Site ADT	2116 Con ADT	nmitted relopment	908	Total Left Approach ADT	3024	Right Approach Site ADT	2722	Committed Development ADT	908	Total Right Approach ADT	3630	-*	100'			2	×	¥	:		2	
Total Number (Does Not Incl	of Throug ude Turn L	h Lanes anes)		2 la	nes	Number of	intersec		з		taper				××			≥				
Roadway Func	tional Class	sification		Major C	ollector	Calculation for (specify leg) Proposed Entrance 2					220		:			10	<u>.</u>		┋╵╴╴			
Left-Approach Projected 10 yr Roadway ADT + Total Site + Committed Development ADT 9778				78	Right-Approach Projected 10 yr Roadway ADT + Total Site + Committed Development ADT 10384						60				<u> </u>	*				Exa		
K Factor				10	.8	D Factor 56.2						\downarrow			\leq			<u> </u>		1	\wedge	m
Left Turr	n Infor	matior	1			Right Turn Information											<u> </u>	*	1			ple
Left Turn VPH				8	9	Right Turn ADT Over 400						+		$\uparrow\uparrow$		•	† Å			- 2 Ac		
Left Turn Appr	roach Grad	le		0.0)%	Right Turn Approach Grade				0.0%				•		190				$\left[\right]$	240 190	cess
Heavy Vehicle	%			5	i	Effective Ra	idius of E	intrance		R≦	50'				l le	40				95		
10 Yr Opposin	g Vol. (Mar	nual Input -	Veh/hr)	c)														1.			
10 Yr Opposin	g Volume	(Calculated)		410 V	eh/hr	Right Turn	Length			240) ft				taper	50					7	
10 Yr Opposing Volume (Calculated Vol.) 410 Veh/hr					eh/hr									Ì					4	ĕ	~	
Left Turn Length 220 ft																		Å				
						1																