

#### **Technical Memorandum**

Date: May 12, 2020 To: Peter Haag

From: Mir Wahed and Joanne Arellano

CC: Chris Sylvester, Max Saintil, Ryan Kusy, Yahya Kenarangi

Project: Pavement and Rehabilitation Design Services

RE: Memorial Drive Before and After Study

Contract No: T201806201 JMT Project No: 17-13205-500

The purpose of this technical memorandum is to review the traffic and safety impacts along Memorial Drive from the US Route 13 intersection to the Delaware Route 9 intersection before and after implementation of the road diet. Memorial Drive was converted from a four-lane section roadway to a two-lane section as part of the *Pavement and Rehabilitation North, 2019 project (DelDOT Contract No. T201806201).* The road diet was implemented on October 18, 2019. This document will summarize before and after results of the road diet installation based on travel time, spot speed, traffic analysis, and crash evaluation.

#### **Background**

Memorial Drive is classified as a minor arterial roadway with an AADT of approximately 9,000 vehicles per day and a posted speed limit of 35 miles per hour. Memorial Drive is divided by a concrete median, terminates to the east with its intersection with Delaware Route 9 and terminates to the west with its intersection with US Route 13. Prior to the road diet, within this segment, Memorial Drive was a four-lane roadway. The study area is surrounded primarily by residential uses and four roadways (Lind Avenue, Bizarre Drive, Karlyn Drive, and Parma Avenue) intersect with Memorial Drive to form five unsignalized intersections (Karlyn Drive intersects with Memorial Drive twice). DART Bus Route 14 also traverses along the roadway and has stops at each of those unsignalized intersections. Driveways to single family homes, on-street parking, and sidewalks exist along both sides of the roadway.

As part of the Memorial Drive Pavement and Rehabilitation design service, DelDOT implemented a road diet which converted the roadway from a four-lane section to a two-lane section to improve safety for pedestrians seeking to cross the street as well as a five-foot bike lane and nine-foot curbside parking in each direction. The Before and After study has been conducted to evaluate the operational and safety impacts of the road diet. The before study results contained in this memorandum are taken from the June 18, 2018 Memorial Drive Technical Memorandum prepared by JMT.

#### **Capacity Analysis Methodology**

Traffic observations along the corridor were conducted on Tuesday, May 7, 2019 which was prior to the construction of the road diet and during a typical weekday morning, afternoon and evening peak period when all schools were in session. The maximum queue lengths along each approach at the study intersections from the May 7, 2019 observations were compared to the queue results from the June 18, 2018 Memorial





Drive Traffic Study Technical Memorandum and were found to be consistent with each other. As such, the before study results are from the June 18, 2018 Memorial Drive Traffic Study Technical Memorandum.

Traffic counts and observations for the after study were conducted on Thursday, January 16, 2020 which was after the completion of the road diet construction and during a typical weekday morning, afternoon and evening peak period when all schools were in session. Data and traffic observations were collected during the afternoon peak period as this is the time period when schools in the area are dismissed. Figures summarizing the volumes utilized in the before and after study are included in Appendix A. It should be noted that the Delaware Route 9/Halcyon Drive intersection was closed due to construction during the January 2020 traffic counts with traffic detoured to utilize the Memorial Drive/Lind Avenue intersection to access Halcyon Drive. As such, the traffic volumes marked by an asterisk were based on the volumes from the June 18, 2018 Memorial Drive Traffic Study Technical Memorandum.

Synchro software was utilized to conduct the traffic analysis. Appendix B summarizes the AM and PM peak hour delay and queue results for before and after implementation of the road diet. The afternoon peak period was not analyzed as the PM peak period contained higher volumes.

During the AM and PM peak periods the study intersections operate at acceptable Level of Service (LOS) C or better before implementation of the road diet and at acceptable LOS D or better after implementation of the road diet. Queue lengths at the unsignalized intersections are minimal before and after construction. Specifically, the calculated 95<sup>th</sup> percentile queue lengths along the minor streets are approximately 20 feet before implementation and approximately 30 feet after implementation. Please see Appendix B for additional information. The queue tables within Appendix B also include a comparison between the observed queues from field observations and the calculated queues from the Synchro results. Based on the comparison, most of the observed queues at the unsignalized intersections were found to be longer than the calculated queues but were noted to dissipate quickly without impacting operations along any adjacent intersections.

At the signalized intersection of US Route 13 with Memorial Drive, the calculated 95<sup>th</sup> percentile queue lengths are comparable between the before and after implementation conditions. However, at the signalized intersection of Delaware Route 9 with Memorial Drive, queue lengths were increased along the eastbound and westbound Memorial Drive left turn/through lanes as well as along the northbound Delaware Route 9 left turn lane. These increases were also consistent with the observed queue lengths. The increased queue length along the northbound Delaware Route 9 left turn lane could be accommodated within the provided storage length and the increased queue length along the eastbound Memorial Drive left turn/through lane would not spillback onto the Lind Avenue intersection. The increased queue length along the westbound Memorial Drive left turn/through lane would spillback past the motel entrance.

#### **Travel Time Study**

JMT collected travel times along the Memorial Drive corridor, from US Route 13 to the Delaware Route 9 intersections in each direction during the weekday morning, afternoon, and evening peak periods. Table 1 summarizes the travel times for before and after implementation of the road diet. The before implementation travel times were collected on Tuesday, May 7, 2019 and the after-implementation travel times were collected



on Thursday, January 9, 2020 and Thursday, January 16, 2020. Schools were in session during the data collection.

**Table 1: Travel Time Results** 

Beginning Intersection	Ending Intersection	Peak Period	Before Implementation Time (sec)	After Implementation Time (sec)
		AM	98	104
Memorial Drive/Delaware Route 9	Marsarial Drive /US Davita 12	Afternoon	95	102
Wiemonal Drive/ Delaware Route 9	Memorial Drive/US Route 13	PM	96	105
		Average	96	104
		AM	98	103
Memorial Drive/US Route 13	Memorial Drive/Delaware Route 9	Afternoon	98	103
iviemonal brive/03 Route 13	Wiemonai Drive, Delaware Route 9	PM	99	102
		Average	98	103

#### **Speed Study**

JMT conducted a spot speed study along eastbound and westbound Memorial Drive in accordance with the Institute of Transportation Engineers (ITE) standards for before and after implementation of the road diet. The spot speed study was performed at one location between the Parma Avenue and Karlyn Drive intersections utilizing ATR (Automatic Traffic Recorder) devices. The posted speed limit along this section of roadway is 35 miles per hour.

The before study was based on data collected from Tuesday, May 7, 2019 to Friday, May 17, 2019 during the hours of 10:00 a.m. to 2:00 p.m. when traffic was more free flowing. Based on the data, the 85<sup>th</sup> percentile speed before implementation of the road diet was 45 miles per hour. Due to a malfunction with the ATR equipment during the January 2020 data collection efforts and the current Covid-19 situation, speed data could not be collected for the after study. However, as the travel time results showed a slight increase in travel time it could be expected that there would be a slight decrease in speed. Updated speed data information may be collected once traffic patterns appear more typical.

#### **NCHRP 562 - Midblock Crossing Evaluation**

Per the June 18, 2018 Memorial Drive Traffic Study Technical Memorandum, "crosswalk" pedestrian treatments were recommended at the Memorial Drive intersections with Karyln Drive (west), Parma Avenue, Karlyn Drive (east), Lind Avenue, and Bizarre Drive under 2018 conditions with or without the provision of a median for a two-stage crossing and utilizing the 35 miles per hour travel speed consistent with the posted speed limit. A "crosswalk" treatment as defined in NCHRP Report 562 is a marked crosswalk with just pavement markings. For the final design with the implementation of the road diet, crosswalks were installed at the Karlyn Drive (west), Parma Avenue, Karlyn Drive (east), and Bizarre Drive intersections and two-stage crossings were not provided.



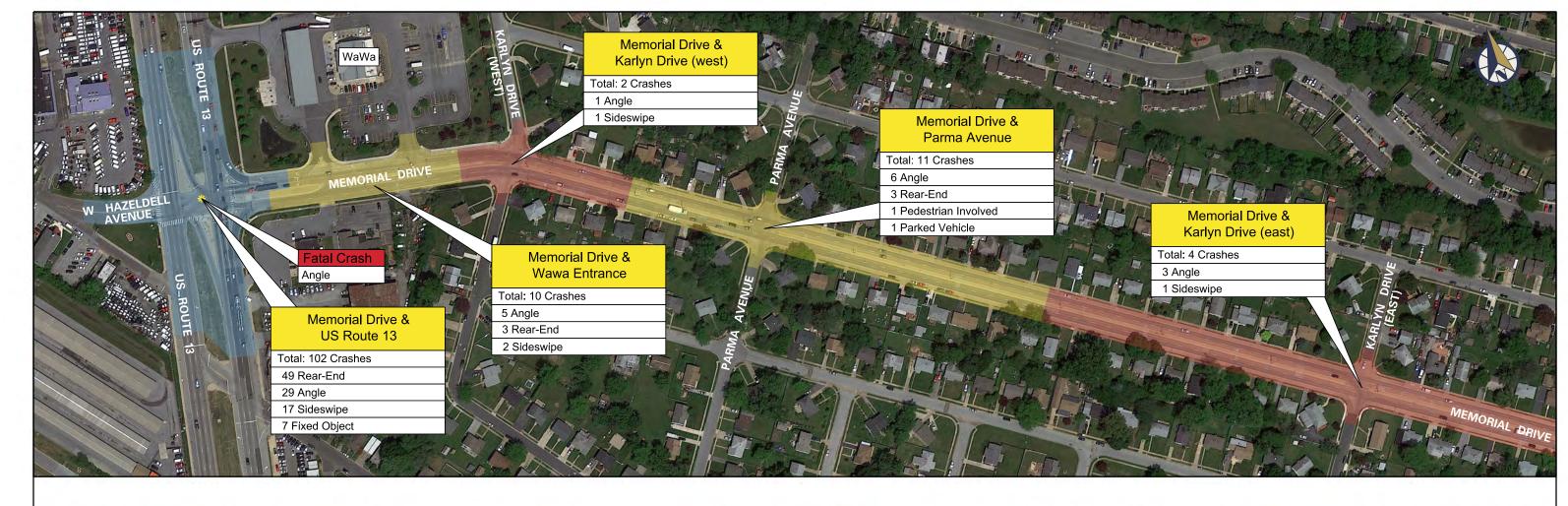
#### **Memorial Drive Before and After Study**

Utilizing the after implementation volumes, the crossing distance from the road diet and a 35 miles per hour travel speed consistent with the posted speed limit, "crosswalk" pedestrian treatments are recommended at each unsignalized intersection without the provision of a two-stage crossing. An additional analysis was conducted utilizing a 45 miles per hour travel speed consistent with the speed study results and "active or enhanced" pedestrian treatments are recommended at each unsignalized intersection. An "active or enhanced" treatment is defined in NCHRP Report 562 as a device that enhances the visibility of the crossing location and pedestrians and include warning signs, pavement markings, in-roadway warning lights, or overhead flashing amber beacons. For the final design, pedestrian warning and school crossing signs at and ahead of the crosswalks were recommended and were confirmed to be installed in the field providing "enhanced" treatments. Appendix C contains the NCHRP Report 562 worksheets completed for each intersection as part of the after-implementation evaluation.

#### **Crash Evaluation**

In the June 18, 2018 Memorial Drive Traffic Study Technical Memorandum, JMT reviewed crash data along Memorial Drive from the US Route 13 to the Delaware Route 9 intersections from April 2015 to April 2018. As summarized on Figure 1, a total of 205 crashes during the three-year study period were reported. Out of the 205 crashes, 37 crashes occurred in the study area from the Wawa Entrance to Lind Avenue (excludes the incidents reported at the US Route 13 and Delaware Route 9 signalized intersections).

For the crash evaluation after implementation of the road diet JMT reviewed crash data from October 2019 to February 2020. As summarized on Figure 2, a total of 23 crashes during the four-month study period were reported. Out of the 23 crashes, 6 crashes occurred in the study area from the Wawa Entrance to Lind Avenue and one was a fatal crash which involved a southbound vehicle at the Karlyn Drive (east) intersection who failed to yield to right-of-way with a vehicle travelling westbound on Memorial Drive.





CRASH DATA APRIL 2015 – APRIL 2018





CRASH DATA OCTUBER 2019 – FEBRUARY 2020



Table 2 presents the crash summary by manner of impact type before and after implementation of the road diet along Memorial Drive in the study area from the Wawa Entrance to Lind Avenue. As depicted on Table 2, sideswipe crashes as well as incidents with parked vehicles and pedestrians were not reported during the after-implementation crash study period. Appendix D contains the crash data summary tables.

Table 2:

Crash Summary by Manner of Impact Type

Memorial Drive, from Wawa Entrance to Lind Avenue

	Number of Crashes (percentages within study area)				
Manner of Impact	Before Implementation (Based on 3 years of crash data)	After Implementation (Based on 4 months of crash data)			
Angle	19 (51%)	1 (17%)			
Rear-End	9 (24%)	4 (66%)			
Sideswipe	4 (11%)	-			
Out of Control*	2 (6%)	1 (17%)			
Parked Vehicle	2 (6%)	-			
Pedestrian Involved	1 (2%)	-			
Total	37 (100%)	6 (100%)			

<sup>\*</sup>The out-of-control incidents were due to drivers losing control of their own vehicles as a result of either speeding, a health condition, or other environmental conditions. These out-of-control incidents resulted in a collision with either the median or a tree.

#### Gap Study

JMT conducted a gap study on Thursday, January 16, 2020 during the morning, afternoon, and evening peak hour at the Memorial Drive intersection with Karlyn Drive (east). The purpose of this study is to determine the availability of appropriate left turn movement gaps from southbound Karlyn Drive (east) during the highest volume peak period. Based on the *Highway Capacity Manual 6<sup>th</sup> Edition*, Exhibit 20-12 lists the critical gap for a left turn and a through movement from a minor street with two lanes along the major street is 7.1 seconds and 6.5 seconds, respectively. There were 118, 162, and 144 gaps that were 7.1 seconds or longer in duration during the morning, afternoon and evening peak hours, respectively. Appendix E summarizes the gap results.

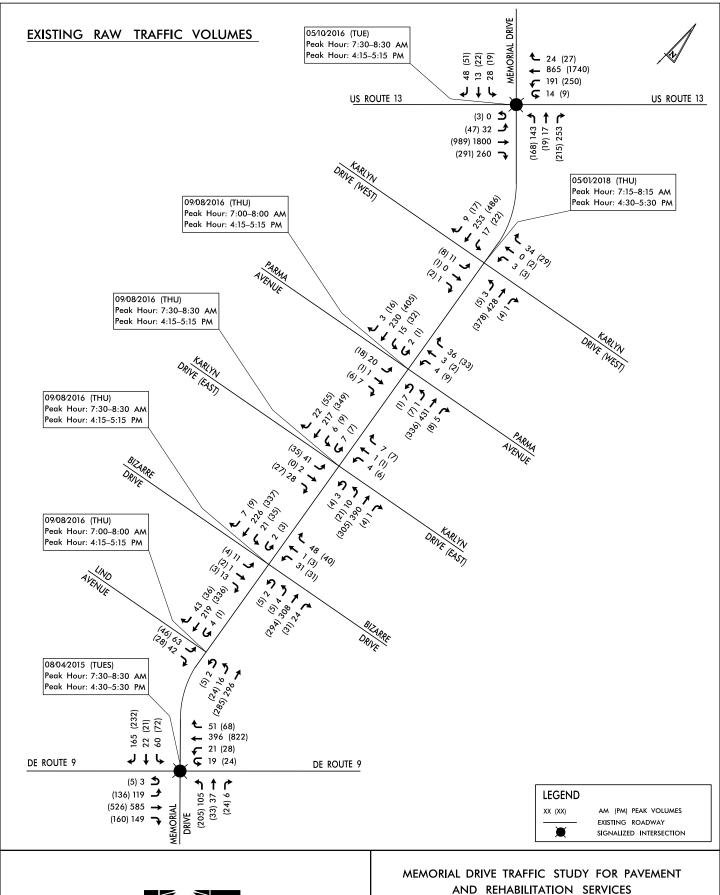


APPENDIX A Volume Figures



### **Before Study**

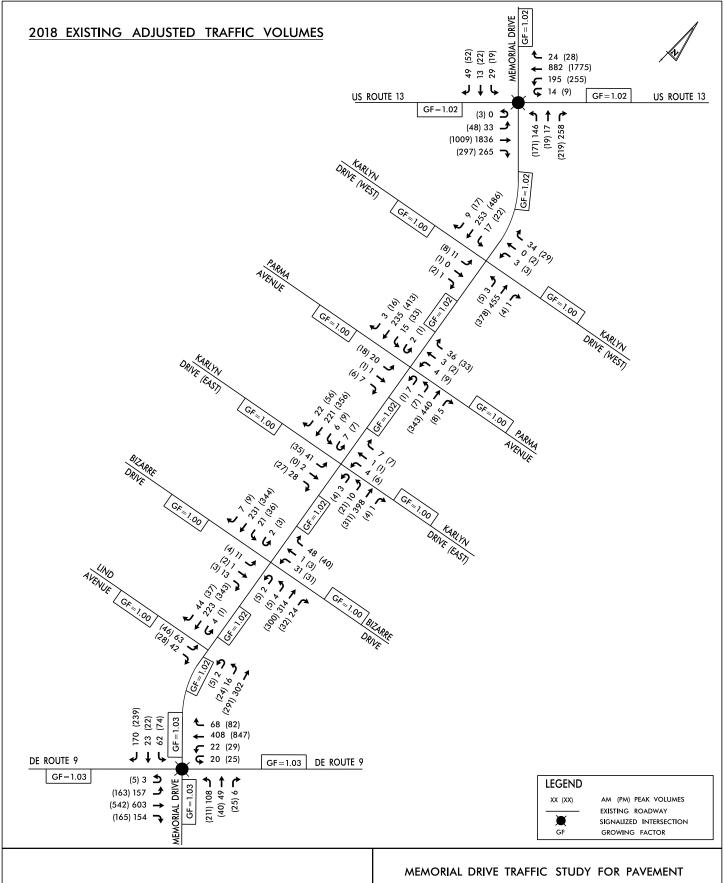
\*From the June 18, 2018 Technical Memorandum for the Memorial Drive Traffic Study prepared by JMT





NEW CASTLE COUNTY, DELAWARE

N.T.S FIGURE A-1 **JUNE 2018** 





AND REHABILITATION SERVICES

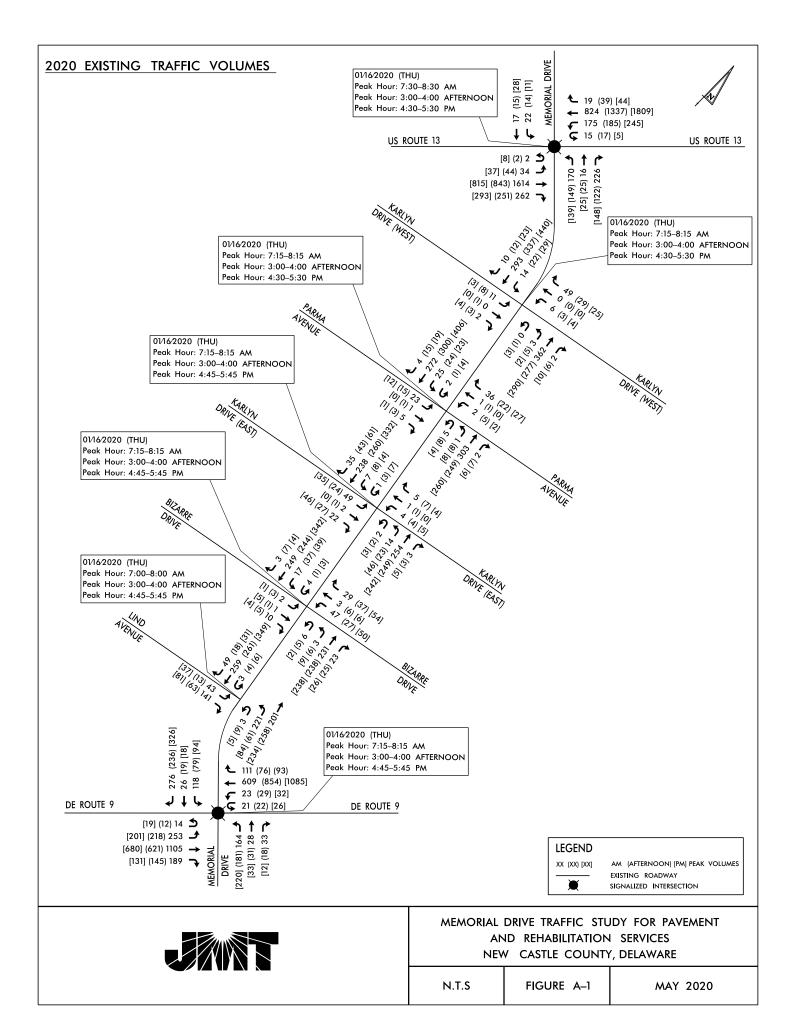
NEW CASTLE COUNTY, DELAWARE

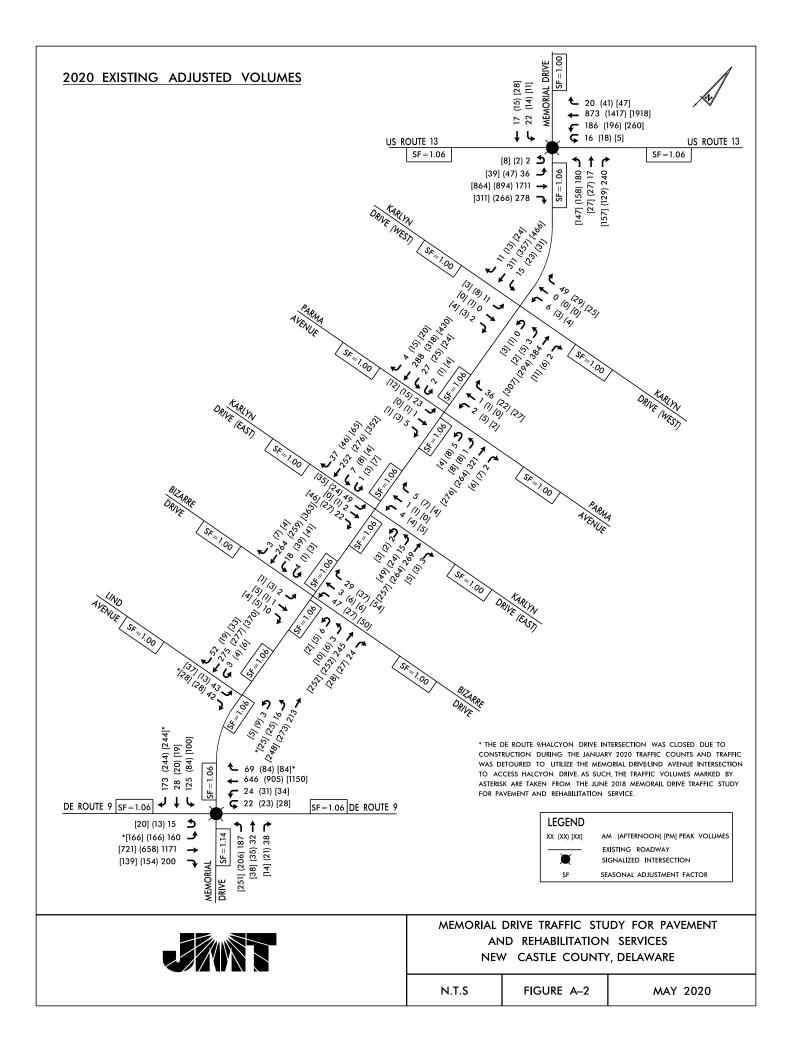
N.T.S FIGURE A-2

JUNE 2018



**After Study** 







APPENDIX B
Capacity Analysis Results

#### Signalized Intersection LOS

Intersection	Peak Hour	2018 Bef	ore Study	2020 Aft	er Study
	Hour	LOS	Delay (sec)	LOS	Delay (sec)
Memorial Drive/US Route 13	AM	С	30.0	D	36.3
Memorial Drive/03 Route 13	PM	С	26.7	С	27.5
Memorial Drive/Delaware Route 9	AM	С	21.1	С	32.8
Memorial Drive/Delaware Route 9	PM	С	28.3	D	36.1

Before LOS (Delay) based on HCM 2010 methodology.

After study LOS (Delay) based on HCM 6th edition methodology.

2018 Before Study results are from the June 18,2018 Technical Memorandum for the Memorial Drive Traffic Study prepared by JMT.

95th Percentile Signalized Queue Results

Intersection	Peak Hour	Movement	Storage		ore Study e (feet)	2020 After Study Queue (feet)	
	r cun riou:		(feet)	Observed Queue	Calculated Queue	Observed Queue	Calculated Queue
		EBL Memorial Drive	-	75	45	50	37
	AM	WBL Memorial Drive	-	125	121	150	148
	Alvi	NBL US Route 13	140	0	26	75	38
Memorial Drive/US Route		SBL US Route 13	190	150	#321	150	#381
13		EBL Memorial Drive	-	25	38	50	23
	PM	WBL Memorial Drive	-	50	132	75	#158
	FIVI	NBL US Route 13	150	50	39	75 75	40
		SBL US Route 13	150	250	#380	250	#366
		EBLT Memorial Drive	-	125	74	150	236
	AM	WBLT Memorial Drive	-	100	#182	175	315
Memorial Drive/Delaware	AIVI	NBL Delaware Route 9	140	25	112	75	134
Route 9		SBL Delaware Route 9	190	50	38	50	44
		EBLT Memorial Drive	-	75	96	225	174
	PM	WBL Memorial Drive	-	150	#308	325	#465
	FIVI	NBL Delaware Route 9	150	75	118	300	#224
		SBL Delaware Route 9	150	75	40	75	57

#### Notes:

m Volume for 95th percentile queue is metered by upstream signal.

Observed queues are from field observations.

 ${\it Calculated\ queues\ are\ 95th\ percentile\ queue\ lengths\ based\ on\ Synchro\ methodology}.$ 

# 95th percentile volume exceeds capacity, queue may be longer.

 $2018\ Before\ Study\ results\ are\ from\ the\ June\ 18,2018\ Technical\ Memorandum\ for\ the\ Memorial\ Drive\ Traffic\ Study\ prepared\ by\ JMT.$ 

#### Unsignalized Intersection LOS

Unsignalized Intersection LOS  Intersection	Peak	Movement	2018 Bei	2018 Before Study		ter Study
	Hour		LOS	Delay (sec)	LOS	Delay (sec)
		EBL Memorial Drive	А	8.5	Α	8.3
	AM	WBL Memorial Drive	А	7.8	Α	8.0
	AIVI	NB Karlyn Drive (west) Approach	В	13.9	С	19.0
Memorial Drive/Karlyn Drive		SB Karlyn Drive (west) Approach	В	10.7	В	12.5
(west)		EBL Memorial Drive	А	8.2	Α	8.0
	PM	WBL Memorial Drive	А	8.5	Α	8.4
	FIVI	NB Karlyn Drive (west) Approach	С	17.8	С	15.7
		SB Karlyn Drive (west) Approach	В	11.2	В	11.9
		EBL Memorial Drive	А	8.5	Α	8.1
	AM	WBL Memorial Drive	А	8.7	Α	7.9
	Alvi	NB Parma Avenue Approach	В	13.2	С	18.0
Memorial Drive/Parma Avenue		SB Parma Avenue Approach	В	11.1	В	11.5
wemonal brive/Parma Avenue		EBL Memorial Drive	А	8.2	Α	8.0
	PM	WBL Memorial Drive	А	8.7	Α	8.6
	PIVI	NB Parma Avenue Approach	С	17.2	С	20.6
		SB Parma Avenue Approach	В	12.2	В	10.9
	АМ	EBL Memorial Drive	А	9.1	Α	8.0
		WBL Memorial Drive	А	8.6	Α	8.1
		NB Karlyn Drive (east) Approach	В	12.9	С	15.5
Memorial Drive/Karlyn Drive		SB Karlyn Drive (east) Approach	В	12.0	В	12.9
(east)		EBL Memorial Drive	А	8.6	Α	7.9
	PM	WBL Memorial Drive	А	8.7	Α	8.5
	PIVI	NB Karlyn Drive (east) Approach	В	14.5	С	17.1
		SB Karlyn Drive (east) Approach	В	13.3	С	16.1
		EBL Memorial Drive	А	8.4	Α	8.2
	AM	WBL Memorial Drive	А	8.3	Α	7.9
	AlVI	NB Bizarre Drive Approach	В	12.0	В	11.3
Mamarial Drive / Birone Drive		SB Bizarre Drive Approach	В	12.2	С	15.8
Memorial Drive/Bizarre Drive		EBL Memorial Drive	А	8.4	Α	8.0
	PM	WBL Memorial Drive	А	8.7	Α	8.1
	PIVI	NB Bizarre Drive Approach	С	15.9	С	15.1
		SB Bizarre Drive Approach	В	13.6	С	17.6
		EBL Memorial Drive	А	4.5	Α	*
	AM	WBL Memorial Drive	А	8.0	Α	8.2
Mamorial Drive / Lind Acces		NB Lind Avenue Approach	В	12.8	В	13.5
Memorial Drive/Lind Avenue		EBL Memorial Drive	А	4.5	Α	*
	PM	WBL Memorial Drive	А	8.6	Α	8.2
		NB Lind Avenue Approach	В	13.2	В	14.1

#### Note:

Before study LOS (Delay) based on HCM 2010 methodology.

After study LOS (Delay) based on HCM 6th edition methodology.

 $2018\ Before\ Study\ results\ are\ from\ the\ June\ 18,2018\ Technical\ Memorandum\ for\ the\ Memorial\ Drive\ Traffic\ Study\ prepared\ by\ JMT.$ 

<sup>\*</sup>LOS (delay) results not provided by software.

Unsignalized Intersection Queue	Peak	Movement		ore Study e (feet)	2020 Aft	ter Study
	Hour		Observed Queue	Calculated Queue		Calculated Queue
		EBL Memorial Drive	0	3	0	0
	AM	WBL Memorial Drive	0	0	0	0
	Alvi	NB Karlyn Drive (west) Approach	25	3	25	5
Memorial Drive/Karlyn Drive		SB Karlyn Drive (west) Approach	0	5	25	10
(west)		EBL Memorial Drive	0	3	25	3
	PM	WBL Memorial Drive	0	0	0	0
	PIVI	NB Karlyn Drive (west) Approach	50	3	0	3
		SB Karlyn Drive (west) Approach	0	5	0	5
		EBL Memorial Drive	0	0	25	3
	AM	WBL Memorial Drive	0	0	0	0
	AIVI	NB Parma Avenue Approach	25	5	75	8
Memorial Drive/Parma Avenue		SB Parma Avenue Approach	25	5	25	5
Memoriai Drive/Parma Avenue		EBL Memorial Drive	0	3	25	3
	PM	WBL Memorial Drive	0	0	0	0
	PIVI	NB Parma Avenue Approach	25	8	Diserved Queue 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5
		SB Parma Avenue Approach	25	8		5
		EBL Memorial Drive	0	0	25	0
		WBL Memorial Drive	0	0	25	0
	AM	NB Karlyn Drive (east) Approach	25	13	25 25 25	18
Memorial Drive/Karlyn Drive		SB Karlyn Drive (east) Approach	25	3	0	3
(east)		EBL Memorial Drive	0	0	0	0
	55.4	WBL Memorial Drive	0	3	0	5
	PM	NB Karlyn Drive (east) Approach	50	13	75	23
		SB Karlyn Drive (east) Approach	25	3	0	3
		EBL Memorial Drive	0	3	25	3
		WBL Memorial Drive	0	0	25	3
	AM	NB Bizarre Drive Approach	0	5	0	0
		SB Bizarre Drive Approach	0	13	50	20
Memorial Drive/Bizarre Drive		EBL Memorial Drive	0	3	25	3
	55.4	WBL Memorial Drive	0	0	0	0
	PM	NB Bizarre Drive Approach	0	3	0	3
		SB Bizarre Drive Approach	50	15	25	30
		EBL Memorial Drive	0	0	0	*
	AM	WBL Memorial Drive	0	0	e Observed Queue  0 0 25 25 25 0 0 0 75 25 25 0 75 25 25 0 0 25 25 0 0 25 25 25 25 25 25 25 25 25 25 25 25 25	3
		NB Lind Avenue Approach	25	18		18
Memorial Drive/Lind Avenue		EBL Memorial Drive	0	0	0	*
	PM	WBL Memorial Drive	0	3	50	3
		NB Lind Avenue Approach	25	13	25	13

#### Notes:

Observed queues are from field observations.

Calculated queues are 95th percentile queue lenghts based on Synchro methodology.

 $2018\ Before\ Study\ results\ are\ from\ the\ June\ 18,2018\ Technical\ Memorandum\ for\ the\ Memorial\ Drive\ Traffic\ Study\ prepared\ by\ JMT.$ 

<sup>\*</sup>Queue results not provided by software.



# APPENDIX C NCHRP 562 Worksheets

Midblock Crossing Evaluation Results

Year	Intersection		sumed Speed Crossing Type		Total Pedestrian Delay		Treatment Warranted	
Evaluated	Evaluated			Distance (Feet)	Hours	Seconds		
	Memorial Drive/Karlyn Drive	≤35	One Stage	47	0.9	3,240	Crosswalk	
	(west)	>35	One Stage	47	3.7	13,320	Active or Enhanced	
	Memorial Drive/Parma Avenue	≤35	One Stage	47	0.7	2,520	Crosswalk	
	Wellonal Drive/Farma Avenue	>35	One Stage	47	2.8	10,080	Active or Enhanced	
2020	Memorial Drive/Karlyn Drive	≤35	One Stage	47	0.7	2,520	Crosswalk	
2020	(east)	>35	One Stage	47	2.1	7,560	Active or Enhanced	
	Memorial Drive/Bizarre Drive	≤35	One Stage	47	0.6	2,160	Crosswalk	
	Wemonal Drive/ Bizarre Drive	>35	One Stage	47	1.9	6,840	Active or Enhanced	
	Memorial Drive/Lind Avenue	≤35	One Stage	47	0.5	1,800	Crosswalk	
	wemonar brive/ Linu Avenue	>35	One Stage	47	1.6	5,760	Active or Enhanced	

This spreadsheet combines Worksheet 1 and Worksheet 2 (Appendix A, pages 69-70) of TCRP Report 112/NCHRP Report 562 (Improving Pedestrian Safety at Unsignalized Intersections) into an electronic format. This spreadsheet should be used in

Key

Conjunction with, and not independent of. Appendix A documentation.

This spreadsheet is still under development, please inform TTI if errors are identified.

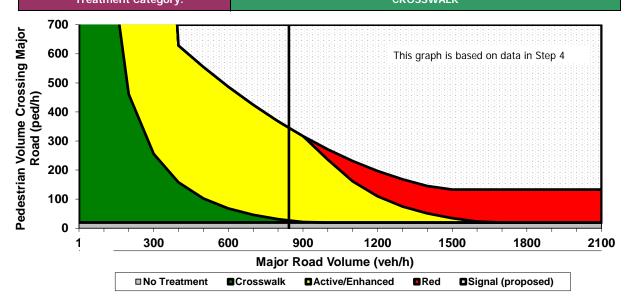
Blue fields contain descriptive information.

Green fields are required and must be completed.

Tan fields are adjustments that are filled out only under certain conditions (follow instructions to the left of the cell).

Gray fields are automatically calculated and should not be edited.

Analyst and Site Information						
Analyst JMT - YK	Major Street Memorial Drive					
Analysis Date April 27, 2020	Minor Street or Location Karlyn Drive (west)					
Data Collection Date January 16, 2020	Peak Hour PM	Peak Hour PM				
tep 1: Select worksheet:						
Posted or statutory speed limit (or 85th percentile speed) on	the major street (mph)	1a	35			
Is the population of the surrounding area <10,000? (enter Y	<b>ES</b> or <b>NO</b> )	1b	no			
tep 2: Does the crossing meet minimum ped	estrian volumes to be considered for a traffic	control de	vice?			
Peak-hour pedestrian volume (ped/h), V <sub>p</sub>		2a	20			
Result: Go to step 3.						
tep 3: Does the crossing meet the pedestrian	n warrant for a traffic signal?					
Major road volume, total of both approaches during peak hou	ır (veh/h), V <sub>maj-s</sub>	<i>3a</i>	844			
[Calculated automatically] Preliminary (before min. threshold)	peak hour pedestrian volume to meet warrant	<i>3b</i>	345			
[Calculated automatically] Minimum required peak hour pede	strian volume to meet traffic signal warrant	<i>3c</i>	345			
Is 15th percentile crossing speed of pedestrians less than 3.5	ft/s (1.1 m/s)? (enter <b>YES</b> or <b>NO</b> )	3d	no			
If 15th percentile crossing speed of pedestrians is less than 3	.5 ft/s % rate of reduction for 3c (up to 50%)	<i>3e</i>				
(1.1 m/s), then reduce $3c$ by up to 50%.	Reduced value or 3c	3f	345			
Result: The signal warrant is not met. Go to step	4.					
tep 4: Estimate pedestrian delay.						
Pedestrian crossing distance, curb to curb (ft), L		4a	47			
Pedestrian walking speed (ft/s), $S_p$ (suggested speed = 3.5 f	ft/s)	4b	3.5			
Pedestrian start-up time and end clearance time (s), $t_{\rm s}$ (sugg	gested start-up time = 3 sec)	4c	3			
[Calculated automatically] Critical gap required for crossing p		4d	16			
Major road volume, total both approaches OR approach being is present, during peak hour (veh/h), $V_{\text{maj-d}}$	g crossed if raised median island	4e	844			
Major road flow rate (veh/s), v		4f	0.23			
Average pedestrian delay (s/person), d <sub>p</sub>		<i>4g</i>	169			
Total pedestrian delay (h), D <sub>p</sub> The value in 4h is the calcul		4h	0.9			
major roadway without a crossing treatment (assumes 0% has been measured at the site, that value can be entered in		4 <i>i</i>	0.005			
	destrian delay and expected motorist compli	ance.				
Expected motorist compliance at pedestrian crossings in region Compliance	on: enter HIGH for High Compliance or LOW for Low	5a	high			
Treatment Category:	CROSSWALK					



This spreadsheet combines Worksheet 1 and Worksheet 2 (Appendix A, pages 69-70) of TCRP Report 112/NCHRP Report 562 (Improving Pedestrian Safety at Unsignalized Intersections) into an electronic format. This spreadsheet should be used in

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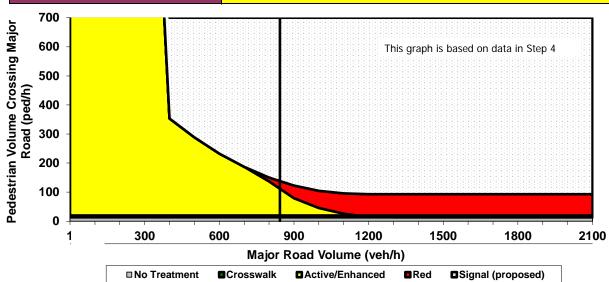
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Gray fields are automatically calculated and should not be edited.

Analyst and Site Information

Analyst and Site Information			
Analyst JMT - YK	Major Street Memorial Drive		
Analysis Date April 27, 2020	Minor Street or Location Karlyn Drive (west)		
Data Collection Date January 16, 2020	Peak Hour PM		
Step 1: Select worksheet:			
Posted or statutory speed limit (or 85th percentile spee	d) on the major street (mph)	1a	45
Is the population of the surrounding area <10,000? (en	ter <b>YES</b> or <b>NO</b> )	1b	no
Step 2: Does the crossing meet minimum	pedestrian volumes to be considered for a traffic	control de	vice?
Peak-hour pedestrian volume (ped/h), V <sub>p</sub>		2a	20
Result: Go to step 3.			
Step 3: Does the crossing meet the pedes	trian warrant for a traffic signal?		
Major road volume, total of both approaches during pea	ık hour (veh/h), V <sub>maj-s</sub>	<i>3a</i>	844
[Calculated automatically] Preliminary (before min. thre	shold) peak hour pedestrian volume to meet warrant	3b	137
[Calculated automatically] Minimum required peak hour	pedestrian volume to meet traffic signal warrant	<i>3c</i>	137
Is 15th percentile crossing speed of pedestrians less that	nn 3.5 ft/s (1.1 m/s)? (enter <b>YES</b> or <b>NO</b> )	3d	no
If 15th percentile crossing speed of pedestrians is less t	han 3.5 ft/s % rate of reduction for 3c (up to 50%)	<i>3e</i>	
(1.1 m/s), then reduce 3c by up to 50%.	Reduced value or 3c	3f	137
Result: The signal warrant is not met. Go to	step 4.		
Step 4: Estimate pedestrian delay.			
Pedestrian crossing distance, curb to curb (ft), L		4a	47
Pedestrian walking speed (ft/s), S <sub>p</sub> (suggested speed =	: 3.5 ft/s)	4b	3.5
Pedestrian start-up time and end clearance time (s), $t_{\mbox{\scriptsize s}}$	. 66	4c	3
[Calculated automatically] Critical gap required for cross		4d	16
Major road volume, total both approaches OR approach is present, during peak hour (veh/h), V <sub>maj-d</sub>	being crossed if raised median island	4e	844
Major road flow rate (veh/s), v		4f	0.33
Average pedestrian delay (s/person), d <sub>p</sub>		<i>4g</i>	666
	calculated estimated delay for all pedestrians crossing the	4h	3.7
major roadway without a crossing treatment (assume has been measured at the site, that value can be ent	4 <i>i</i>	0.021	
Step 5: Select treatment based up on total	I pedestrian delay and expected motorist complia	ance.	
Expected motorist compliance at pedestrian crossings in Compliance	region: enter HIGH for High Compliance or LOW for Low	5a	high
Treatment Category:	ACTIVE OR ENHANCED		



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key

Conjunction with, and not independent of. Appendix A documentation.

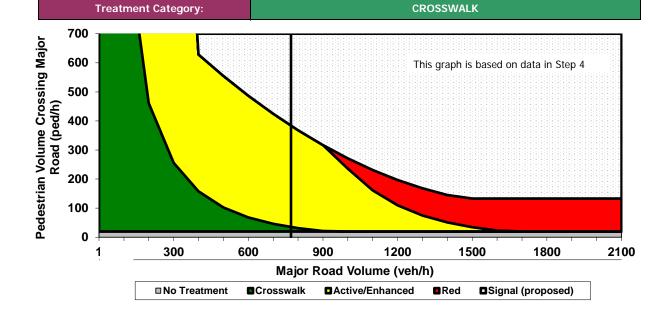
This spreadsheet is still under development, please inform TTI if errors are identified.

Blue fields contain descriptive information.

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	automatically calculated and should no		nis (ioliov	v instructions to the left of the	ie ceii).	
<b>Analyst and Site Inform</b>	nation					
Analyst JMT	T - YK	Majo	or Street	Memorial Drive		
Analysis Date Apri	il 27, 2020	Minor Street or I	Location			
Data Collection Date Janu	uary 16, 2020	Pe	ak Hour	PM		
Step 1: Select workshe	et:					
Posted or statutory speed limit	it (or 85th percentile speed) on the m	najor street (mph)			1a	35
Is the population of the surrou	unding area <10,000? (enter YES or	NO)			1b	no
Step 2: Does the crossing	ing meet minimum pedestri	ian volumes to	o be co	nsidered for a traffic	control de	vice?
Peak-hour pedestrian volume	(ped/h), V <sub>p</sub>				2a	20
Result: Go to step 3.						
Step 3: Does the crossii	ing meet the pedestrian wa	rrant for a tra	iffic sig	ınal?		
Major road volume, total of bo	oth approaches during peak hour (veh	h/h), V <sub>maj-s</sub>			3a	772
[Calculated automatically] Prel	eliminary (before min. threshold) peak	hour pedestrian v	olume to	meet warrant	3b	383
[Calculated automatically] Minimum required peak hour pedestrian volume to meet traffic signal warrant				3с	383	
Is 15th percentile crossing speed of pedestrians less than 3.5 ft/s (1.1 m/s)? (enter <b>YES</b> or <b>NO</b> )				3d	no	
If 15th percentile crossing spe	eed of pedestrians is less than 3.5 ft/s	s % r	ate of red	duction for 3c (up to 50%)	<i>3e</i>	
(1.1 m/s), then reduce $3c$ by		Red	luced valu	ue or <i>3c</i>	3f	383
	rant is not met. Go to step 4.					
Step 4: Estimate pedes						
Pedestrian crossing distance, of	. , ,				4a	47
Pedestrian walking speed (ft/s	s), $S_p$ (suggested speed = 3.5 ft/s)				4b	3.5
Pedestrian start-up time and e	end clearance time (s), t <sub>s</sub> (suggested	start-up time = 3	sec)		4c	3
	tical gap required for crossing pedestr				4d	16
Major road volume, total both is present, during peak hour	approaches OR approach being cross (veh/h), V <sub>maj-d</sub>	sed if raised media	n island		4e	772
Major road flow rate (veh/s), v	v				4f	0.21
Average pedestrian delay (s/pe	person), d <sub>p</sub>				<i>4g</i>	129
	The value in 4h is the calculated e				4h	0.7
major roadway without a crossing treatment (assumes 0% compliance). If the actual total pedestrian delay has been measured at the site, that value can be entered in 4i to replace the calculated value in 4h.					<i>4i</i>	0.004
Step 5: Select treatmer	nt based up on total pedest	rian delay and	d expe	cted motorist complia	ance.	
Expected motorist compliance  Compliance	e at pedestrian crossings in region: en	iter <i>HIGH for Hig</i>	h Compl	liance or LOW for Low	5a	high



This worksheet provides general recommendations on pedestrian crossing treatments to consider at unsignalized intersections; in all cases, engineering judgment should be used in selecting a specific treatment for installation. This worksheet does not apply to school crossings. In addition to the results provided by this worksheet, users should consider whether a pedestrian treatment could present an increased safety risk to pedestrians, such as where there is poor sight distance, complex geometrics, or nearby traffic signals.

Compliance

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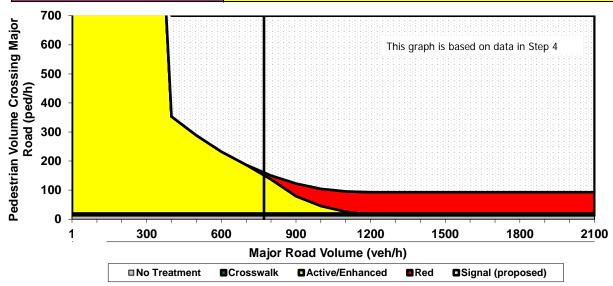
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Analyst and Site Information			
Analyst JMT - YK	Major Street Memorial Drive		
Analysis Date April 27, 2020	Minor Street or Location Parma Avenue		
Data Collection Date January 16, 2020	Peak Hour PM		
Step 1: Select worksheet:			
Posted or statutory speed limit (or 85th percentile speed) of	n the major street (mph)	1a	45
Is the population of the surrounding area <10,000? (enter	YES or NO)	1b	no
Step 2: Does the crossing meet minimum pe	destrian volumes to be considered for a traffic c	ontrol dev	vice?
Peak-hour pedestrian volume (ped/h), V <sub>p</sub>		2a	20
Result: Go to step 3.			
Step 3: Does the crossing meet the pedestri	an warrant for a traffic signal?		
Major road volume, total of both approaches during peak h	our (veh/h), V <sub>maj-s</sub>	<i>3a</i>	772
[Calculated automatically] Preliminary (before min. thresho	d) peak hour pedestrian volume to meet warrant	3b	159
[Calculated automatically] Minimum required peak hour per	destrian volume to meet traffic signal warrant	3c	159
Is 15th percentile crossing speed of pedestrians less than 3	.5 ft/s (1.1 m/s)? (enter <b>YES</b> or <b>NO</b> )	3d	no
If 15th percentile crossing speed of pedestrians is less than	3.5 ft/s % rate of reduction for 3c (up to 50%)	<i>3e</i>	
(1.1 m/s), then reduce $3c$ by up to 50%.	Reduced value or 3c	3f	159
Result: The signal warrant is not met. Go to ste	p 4.		
Step 4: Estimate pedestrian delay.			
Pedestrian crossing distance, curb to curb (ft), L		4a	47
Pedestrian walking speed (ft/s), $S_p$ (suggested speed = 3.	5 ft/s)	4b	3.5
Pedestrian start-up time and end clearance time (s), $t_{\text{s}}$ (su	ggested start-up time = 3 sec)	4c	3
[Calculated automatically] Critical gap required for crossing		4d	16
Major road volume, total both approaches OR approach be is present, during peak hour (veh/h), V <sub>maj-d</sub>	ng crossed if raised median island	4e	772
Major road flow rate (veh/s), v		4f	0.31
Average pedestrian delay (s/person), d <sub>p</sub>		<i>4g</i>	506
	ulated estimated delay for all pedestrians crossing the	4h	2.8
major roadway without a crossing treatment (assumes 0 has been measured at the site, that value can be entered		4i	0.016
Step 5: Select treatment based up on total p	edestrian delay and expected motorist complian	ice.	
Expected motorist compliance at pedestrian crossings in re- Compliance	gion: enter HIGH for High Compliance or LOW for Low	5a	high
Treatment Category:	ACTIVE OR ENHANCED		



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Key

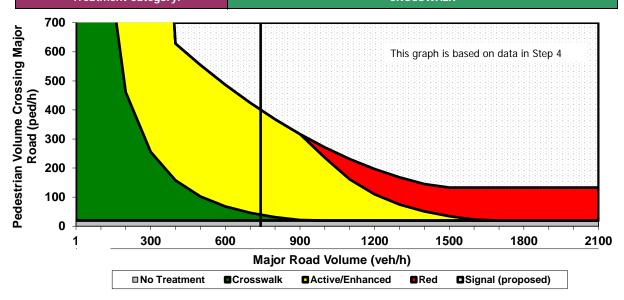
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Analyst and Site Info	rmetion						
			Mada Charat	Managed at Dates			
,	JMT - YK	Min on Chan	•	Memorial Drive			
Analysis Date		Minor Stree	Street or Location Karlyn Drive (east)				
Data Collection Date			Peak Hour	IPM .			
Step 1: Select works							
, , , , , , , , , , , , , , , , , , ,	limit (or 85th percentile spe	, , ,	ıph)		1a	35	
	rrounding area <10,000? (e	•			1b	no	
Step 2: Does the cro		n pedestrian volum	es to be co	nsidered for a traffic			
Peak-hour pedestrian volu	4 7: P				2a	20	
Result: Go to step 3							
Step 3: Does the cro			ı traffic siç	gnal?			
Major road volume, total o	f both approaches during pe	eak hour (veh/h), V <sub>maj-s</sub>			<i>3a</i>	742	
[Calculated automatically]	Preliminary (before min. thr	eshold) peak hour pedestr	ian volume to	meet warrant	3b	400	
[Calculated automatically]	Minimum required peak hou	ır pedestrian volume to me	eet traffic sign	al warrant	3c	400	
Is 15th percentile crossing	speed of pedestrians less th	nan 3.5 ft/s (1.1 m/s)? (e	nter <i>YES</i> or /	<b>VO</b> )	3d	no	
If 15th percentile crossing	speed of pedestrians is less	than 3.5 ft/s	% rate of re	duction for 3c (up to 50%)	<i>3e</i>		
(1.1 m/s), then reduce 30	by up to 50%.		Reduced val	ue or <i>3c</i>	3f	400	
Result: The signal v	warrant is not met. Go to	step 4.	•				
Step 4: Estimate ped	lestrian delay.						
Pedestrian crossing distan	ce, curb to curb (ft), L				4a	47	
Pedestrian walking speed	(ft/s), S <sub>p</sub> (suggested speed	= 3.5 ft/s)			4b	3.5	
Pedestrian start-up time a	nd end clearance time (s), t <sub>s</sub>	(suggested start-up time	= 3 sec)		4c	3	
	Critical gap required for cros				4d	16	
Major road volume, total b is present, during peak h	oth approaches OR approac our (veh/h), V <sub>maj-d</sub>	h being crossed if raised r	nedian island		4e	742	
Major road flow rate (veh/	s), v				4f	0.21	
Average pedestrian delay	(s/person), d <sub>p</sub>				4g	129	
	, D <sub>p</sub> The value in 4h is the				4h	0.7	
	a crossing treatment (assum he site, that value can be en				4 <i>i</i>	0.004	
Step 5: Select treatn	nent based up on tot	al pedestrian delay	and expe	cted motorist compli	ance.		
	nce at pedestrian crossings	in region: enter <i>HIGH for</i>	High Comp	liance or LOW for Low	5a	high	
Compliance					Ja	riigii	
Treatment	Category:			CROSSWALK			



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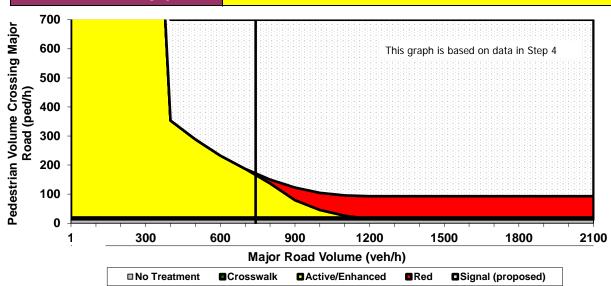
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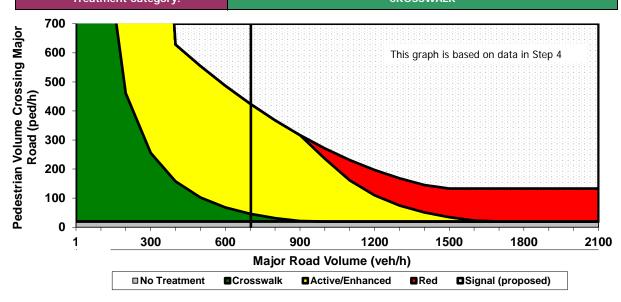
Analyst and Site Information					
Analyst JMT - YK	Major Street Memorial Drive				
Analysis Date April 27, 2020	Minor Street or Location Karlyn Drive (east)				
Data Collection Date January 16, 2020 Peak Hour PM					
Step 1: Select worksheet:					
Posted or statutory speed limit (or 85th percentile	speed) on the major street (mph)	1a	45		
Is the population of the surrounding area <10,000	1b	no			
Step 2: Does the crossing meet minim	um pedestrian volumes to be considered for a traffic	control de	vice?		
Peak-hour pedestrian volume (ped/h), V <sub>p</sub>		2a	20		
Result: Go to step 3.					
Step 3: Does the crossing meet the pe	destrian warrant for a traffic signal?				
Major road volume, total of both approaches during	g peak hour (veh/h), V <sub>maj-s</sub>	3a	742		
[Calculated automatically] Preliminary (before min.	threshold) peak hour pedestrian volume to meet warrant	3b	170		
[Calculated automatically] Minimum required peak	3с	170			
Is 15th percentile crossing speed of pedestrians les	3d	no			
If 15th percentile crossing speed of pedestrians is	ess than 3.5 ft/s % rate of reduction for 3c (up to 50%)	<i>3e</i>			
(1.1 m/s), then reduce 3c by up to 50%.	Reduced value or 3c	3f	170		
Result: The signal warrant is not met. G	o to step 4.				
Step 4: Estimate pedestrian delay.					
Pedestrian crossing distance, curb to curb (ft), L		4a	47		
Pedestrian walking speed (ft/s), S <sub>p</sub> (suggested spe	4b	3.5			
Pedestrian start-up time and end clearance time (s	), t <sub>s</sub> (suggested start-up time = 3 sec)	4c	3		
[Calculated automatically] Critical gap required for	crossing pedestrian (s), t <sub>c</sub>	4d	16		
Major road volume, total both approaches OR appr is present, during peak hour (veh/h), V <sub>maj-d</sub>	oach being crossed if raised median island	4e	742		
Major road flow rate (veh/s), v		4f	0.29		
Average pedestrian delay (s/person), dp		<i>4g</i>	384		
	the calculated estimated delay for all pedestrians crossing the	4h	2.1		
major roadway without a crossing treatment (ass has been measured at the site, that value can be	4 <i>i</i>	0.012			
Step 5: Select treatment based up on	total pedestrian delay and expected motorist complia	ance.			
Expected motorist compliance at pedestrian crossin Compliance	gs in region: enter HIGH for High Compliance or LOW for Low	5a	high		
Treatment Category:	ACTIVE OR ENHANCED				



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nalyst and Site Info				T		
,	JMT - YK		Major Street	Memorial Drive		
Analysis Date		Minor Stree	t or Location	Bizzare Drive		
Data Collection Date		Peak Hour PM				
tep 1: Select works	sheet:					
Posted or statutory speed	limit (or 85th percentile sp	eed) on the major street (m	ph)		1a	35
Is the population of the su	1b	no				
tep 2: Does the cro	ssing meet minimu	m pedestrian volume	es to be co	nsidered for a traffic	control de	vice?
Peak-hour pedestrian volu	me (ped/h), V <sub>p</sub>				2a	20
Result: Go to step :	3.					
tep 3: Does the cro	ssing meet the pede	estrian warrant for a	traffic sig	nal?		
Major road volume, total of	of both approaches during p	oeak hour (veh/h), V <sub>maj-s</sub>			<i>3a</i>	703
[Calculated automatically]	Preliminary (before min. th	reshold) peak hour pedestri	an volume to	meet warrant	<i>3b</i>	423
[Calculated automatically] Minimum required peak hour pedestrian volume to meet traffic signal warrant						423
Is 15th percentile crossing speed of pedestrians less than 3.5 ft/s (1.1 m/s)? (enter YES or NO)				3d	no	
If 15th percentile crossing	speed of pedestrians is les	s than 3.5 ft/s	% rate of re-	duction for 3c (up to 50%)	<i>3e</i>	
(1.1 m/s), then reduce 30	by up to 50%.		Reduced val	ue or <i>3c</i>	3f	423
Result: The signal v	warrant is not met. Go	to step 4.				
tep 4: Estimate ped	destrian delay.					
Pedestrian crossing distan	ce, curb to curb (ft), L				4a	47
Pedestrian walking speed (ft/s), S <sub>p</sub> (suggested speed = 3.5 ft/s)						3.5
Pedestrian start-up time a	nd end clearance time (s),	t <sub>s</sub> (suggested start-up time	= 3 sec)		4c	3
	Critical gap required for cr				4d	16
Major road volume, total be is present, during peak h		ich being crossed if raised m	nedian island		4e	703
Major road flow rate (veh/	's), v				4f	0.20
Average pedestrian delay	(s/person), d <sub>p</sub>				<i>4g</i>	112
		ne calculated estimated dela			4h	0.6
major roadway without a crossing treatment (assumes 0% compliance). If the actual total pedestrian delay has been measured at the site, that value can be entered in 4i to replace the calculated value in 4h.					4 <i>i</i>	0.003
				cted motorist compli	ance.	
Expected motorist compliance  Compliance	ince at pedestrian crossings	s in region: enter <i>HIGH for</i>	High Comp	liance or LOW for Low	5a	high
Treatment	: Category:			CROSSWALK		



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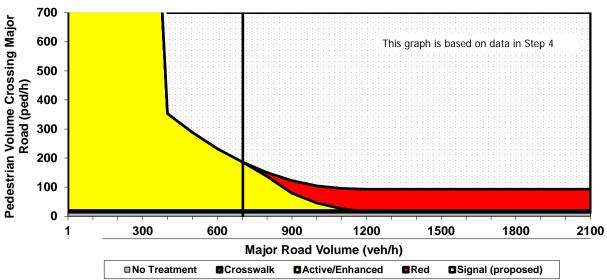
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Analysis Date April 27, 2020	Major Street Me Minor Street or Location Biz		
Data Collection Date January 16, 2020	Peak Hour PM		
itep 1: Select worksheet:	r san risar [rin		
Posted or statutory speed limit (or 85th percentile speed)	on the major street (mph)		<i>1a</i> 45
Is the population of the surrounding area <10,000? (ente		<i>1b</i> no	
tep 2: Does the crossing meet minimum p	•	idered for a traffic con	trol device?
Peak-hour pedestrian volume (ped/h), V <sub>p</sub>			<i>2a</i> 20
Result: Go to step 3.			
tep 3: Does the crossing meet the pedestr	ian warrant for a traffic signa	ıl?	
Major road volume, total of both approaches during peak	hour (veh/h), V <sub>maj-s</sub>		<i>3a</i> 703
[Calculated automatically] Preliminary (before min. thresh	old) peak hour pedestrian volume to me	et warrant	<i>3b</i> 186
[Calculated automatically] Minimum required peak hour p	varrant varrant	<i>3c</i> 186	
Is 15th percentile crossing speed of pedestrians less than	3.5 ft/s (1.1 m/s)? (enter <b>YES</b> or <b>NO</b> )		<i>3d</i> no
If 15th percentile crossing speed of pedestrians is less that	n 3.5 ft/s % rate of reduct	tion for 3c (up to 50%)	<i>3e</i>
(1.1 m/s), then reduce 3c by up to 50%.	Reduced value of	or <i>3c</i>	<i>3f</i> 186
Result: The signal warrant is not met. Go to state 4: Estimate pedestrian delay.	ep 4.		
Pedestrian crossing distance, curb to curb (ft), L			<i>4a</i> 47
Pedestrian walking speed (ft/s), S <sub>D</sub> (suggested speed = 3		4b 3.5	
Pedestrian start-up time and end clearance time (s), $t_s$ (s		4c 3.5	
[Calculated automatically] Critical gap required for crossin	30 i ,		4d 16
Major road volume, total both approaches OR approach b			4e 703
is present, during peak hour (veh/h), V <sub>maj-d</sub>			
Major road flow rate (veh/s), v			<i>4f</i> 0.28
Average pedestrian delay (s/person), d <sub>p</sub>			<i>4g</i> 335
Total pedestrian delay (h), D <sub>p</sub> The value in 4h is the ca major roadway without a crossing treatment (assumes	culated estimated delay for all pedestria		<i>4h</i> 1.9
has been measured at the site, that value can be entered			<i>4i</i> 0.010
tep 5: Select treatment based up on total			).
Expected motorist compliance at pedestrian crossings in r <i>Compliance</i>	egion: enter <i>HIGH for High Complian</i>	ace or LOW for Low	5a high
Treatment Category:	ACTIVE	OR ENHANCED	



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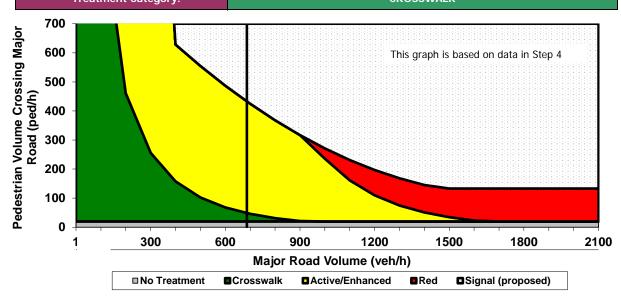
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nalyst and Site Info						
,	JMT - YK		,	Memorial Drive		
Analysis Date		Minor Street	or Location	Lind Avenue		
Data Collection Date			Peak Hour	PM		
tep 1: Select works	sheet:					
		eed) on the major street (mp	oh)		1a	35
	rrounding area <10,000? (				1b	no
tep 2: Does the cro	ssing meet minimur	m pedestrian volume	s to be co	nsidered for a traffic	control de	vice?
Peak-hour pedestrian volu	me (ped/h), V <sub>p</sub>				2a	20
Result: Go to step :	3.					
		estrian warrant for a	traffic sig	nal?		
Major road volume, total o	of both approaches during p	oeak hour (veh/h), V <sub>maj-s</sub>			<i>3a</i>	687
[Calculated automatically]	Preliminary (before min. th	reshold) peak hour pedestri	an volume to	meet warrant	<i>3b</i>	432
[Calculated automatically] Minimum required peak hour pedestrian volume to meet traffic signal warrant						432
Is 15th percentile crossing speed of pedestrians less than 3.5 ft/s (1.1 m/s)? (enter YES or NO)					3d	no
If 15th percentile crossing	speed of pedestrians is les	s than 3.5 ft/s	% rate of red	duction for 3c (up to 50%)	<i>3e</i>	
(1.1 m/s), then reduce 30	by up to 50%.		Reduced value	ue or <i>3c</i>	<i>3f</i>	432
Result: The signal v	warrant is not met. Go t	to step 4.				
tep 4: Estimate ped	destrian delay.					
Pedestrian crossing distan	ce, curb to curb (ft), L				<i>4a</i>	47
Pedestrian walking speed (ft/s), S <sub>p</sub> (suggested speed = 3.5 ft/s)						3.5
Pedestrian start-up time and end clearance time (s), t <sub>s</sub> (suggested start-up time = 3 sec)					4c	3
	Critical gap required for cre				4d	16
Major road volume, total but is present, during peak h		ich being crossed if raised m	edian island		4e	687
Major road flow rate (veh/	′s), v				4f	0.19
Average pedestrian delay (s/person), d <sub>p</sub>						98
		ne calculated estimated delay			4h	0.5
major roadway without a crossing treatment (assumes 0% compliance). If the actual total pedestrian delay has been measured at the site, that value can be entered in 4i to replace the calculated value in 4h.					4 <i>i</i>	0.003
				cted motorist compli	ance.	
Expected motorist compliance  Compliance	ince at pedestrian crossings	s in region: enter <i>HIGH for</i>	High Compi	liance or LOW for Low	5a	high
Treatment	: Category:			CROSSWALK		



This spreadsheet combines Worksheet 1 and Worksheet 2 (Appendix A, pages 69-70) of TCRP Report 112/NCHRP Report 562 (Improving Pedestrian Safety at Unsignalized Intersections) into an electronic format. This spreadsheet should be used in

Key

Conjunction with, and not independent of. Appendix A documentation.

This spreadsheet is still under development, please inform TTI if errors are identified.

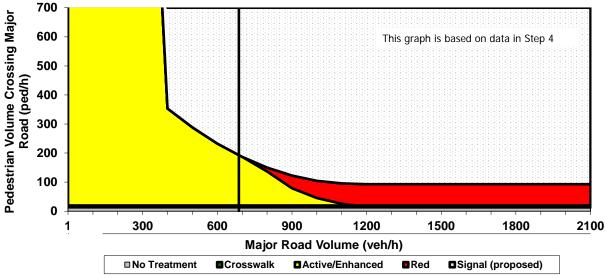
Blue fields contain descriptive information.

Green fields are required and must be completed.

Tan fields are adjustments that are filled out only under certain conditions (follow instructions to the left of the cell).

Gray fields are automatically calculated and should not be edited.

,	formation					
Analys	t JMT - YK		Major Street	Memorial Drive		
	e April 27, 2020		t or Location			
	e January 16, 2020	Willion Street	Peak Hour			
Step 1: Select work			T Cak Tiour	ji ivi		
Posted or statutory spec	ed limit (or 85th percentile speed) o	on the major street (m	ph)		1a	45
Is the population of the	surrounding area <10,000? (enter	YES or NO)			1b	no
tep 2: Does the ci	rossing meet minimum pe	destrian volume	es to be co	nsidered for a traffic	control de	vice?
Peak-hour pedestrian vo	lume (ped/h), V <sub>p</sub>				2a	20
Result: Go to ste	o 3.					
Step 3: Does the co	rossing meet the pedestria	an warrant for a	traffic sig	ınal?		
Major road volume, tota	I of both approaches during peak h	nour (veh/h), V <sub>maj-s</sub>			3a	687
[Calculated automatical]	y] Preliminary (before min. threshol	ld) peak hour pedestri	an volume to	meet warrant	3b	192
[Calculated automaticall	y] Minimum required peak hour peo	destrian volume to me	et traffic sign	al warrant	3с	192
Is 15th percentile crossi	ng speed of pedestrians less than 3	3.5 ft/s (1.1 m/s)? (en	nter <i>YES</i> or <i>N</i>	<i>IO</i> )	3d	no
If 15th percentile crossi	ng speed of pedestrians is less than	1 3.5 ft/s	% rate of red	duction for 3c (up to 50%)	<i>3e</i>	
(1.1 m/s), then reduce	<i>3c</i> by up to 50%.		Reduced value	ue or <i>3c</i>	3f	192
Result: The signa	I warrant is not met. Go to ste	p 4.	•		·	
Step 4: Estimate p	edestrian delay.					
Pedestrian crossing dista	Pedestrian crossing distance, curb to curb (ft), L				<i>4a</i>	47
Pedestrian walking speed (ft/s), S <sub>p</sub> (suggested speed = 3.5 ft/s)					4b	3.5
Pedestrian start-up time and end clearance time (s), t <sub>s</sub> (suggested start-up time = 3 sec)						3
[Calculated automatically] Critical gap required for crossing pedestrian (s), $t_c$						16
Major road volume, tota is present, during peak	I both approaches OR approach bei c hour (veh/h), V <sub>maj-d</sub>	ing crossed if raised m	nedian island		4e	687
Major road flow rate (ve	h/s), v				4f	0.27
Average pedestrian dela	y (s/person), d <sub>p</sub>				<i>4g</i>	293
Total pedestrian delay (					4h	1.6
	at a crossing treatment (assumes 09) t the site, that value can be entered				4i	0.009
	tment based up on total p				ance.	
Expected motorist comp Compliance	liance at pedestrian crossings in req	gion: enter <i>HIGH for</i>	High Comp	liance or LOW for Low	5a	high
	nt Category:		ACT	IVE OR ENHANCED		





**APPENDIX D Crash Evaluation** 



### **Before Study**

\*From the June 18, 2018 Technical Memorandum for the Memorial Drive Traffic Study prepared by JMT

# **Crash Summary Memorial Drive & Lind Avenue**

	Fatality	Injury	Manner of Impact	Lighting Condition	SC	PCC
1	0	1	Front to Rear	Dark-Lighted	Wet	Driver Inattention
2	0	0	Angle	Dark-Not Lighted	Dry	Driving Under The Influence
3	0	0	Angle	Daylight	Wet	Driver Inattention
4	0	1	Angle	Dark-Lighted	Dry	Failed to Yield Right of Way

## **Crash Summary Memorial Drive & Bizarre Drive**

	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC
1	0	1	Out of Control	Dark-Lighted	Dry	Speeding
2	0	1	Front to Rear	Daylight	Dry	Failed to Yield Right of Way
3	0	0	Angle	Dark-Lighted	Dry	Failed to Yield Right of Way
4	0	0	Front to Rear	Dark-Lighted	Dry	Made Improper Turn
5	0	2	Out of Control	Dark-Lighted	Dry	Made Improper Turn
6	0	0	Parked Vehicle	Unknown	Dry	Made Improper Turn

### **Crash Summary Memorial Drive & Karlyn Drive (east)**

	Fatality	Injury	Manner of Impact	Lighting Condition	SC	PCC
1	0	2	Angle	Daylight	Dry	Passed Stop Sign
2	0	0	Angle	Daylight	Dry	Passed Stop Sign
3	0	0	Angle	Daylight	Dry	Driver Inattention
4	0	0	Sideswipe, Same Direction	Daylight	Dry	Failed to Yield Right of Way

# **Crash Summary Memorial Drive & Parma Avenue**

	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC			
1	0	1	Angle	Daylight	Dry	Failed to Yield Right of Way			
2	0	0	Angle	Dark-Lighted	Dry	Failed to Yield Right of Way			
3	0	0	Angle	Dusk	Wet	Failed to Yield Right of Way			
4	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way			
5	0	1	Angle	Dark-Lighted	Dry	Failed to Yield Right of Way			
6	0	2	Front to Rear	Dark-Lighted	Wet	Driver Inattention			
7	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way			
8	0	0	Front to Rear	Daylight	Dry	Following too close Following too close			
9	0	1	Pedestrian Involved	Dark-Lighted	Dry	Made Improper Turn			
10	0	0	Front to Rear	Dark-Lighted	Dry	Driving Under The Influence			
11	0	0	Parked Vehicle	Unknown	Unknown	Made Improper Turn			

<b>Crash Summar</b>	y Memorial	Drive & Karl	yn Drive	(west)
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	Fatality	Injury	Manner of Impact	Lighting Condition	SC	PCC	
1	0	0	Angle	Unknown	Dry	Failed to Yield Right of Way	
2	0	1	Sideswipe, Same Direction	Daylight	Dry	Improper Lane Change	

## **Crash Summary Memorial Drive & Wawa Entrance**

	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC			
1	0	0	Sideswipe, Same Direction	Daylight	Wet	Failed to Yield Right of Way			
2	0	0	Front to Rear	Daylight	Dry	Following too close			
3	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way			
4	0	1	Angle	Daylight	Dry	Made Improper Turn			
5	0	0	Front to Rear	Daylight	Dry	Failed to Yield Right of Way			
6	0	0	Front to Rear	Daylight	Dry	Driver Inattention			
7	0	0	Sideswipe, Same Direction	Daylight	Ice/Frost	Made Improper Turn			
8	0	0	Angle	Daylight	Dry	Made Improper Turn			
9	0	1	Angle	Dark-Lighted	Dry	Failed to Yield Right of Way			
10	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way			

#	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC
1	0	0	Front to Rear	Daylight	Dry	Driver Inattention
2	0	0	Front to Rear	Daylight	Dry	Driver Inattention
3	0	0	Front to Rear	Daylight	Dry	Following Too Close
4	0	2	Front to Rear	Dark-Lighted	Dry	Driver Inattention
5	0	0	Front to Rear	Daylight	Dry	Failed to Yield Right of Way
6	0	0	Front to Rear	Dark-Lighted	Dry	Driver Inattention
7	0	0	Front to Rear	Daylight	Dry	Following Too Close
8	0	0	Front to Rear	Dark-Lighted	Dry	Driver Inattention
9	0	0	Front to Rear	Daylight	Wet	Failed to Yield Right of Way
10	0	0	Front to Rear	Dark-Lighted	Dry	Driving Under The Influence
11	0	1	Front to Rear	Daylight	Dry	Driver Inattention
12	0	0	Front to Rear	Dusk	Wet	Driver Inattention
13	0	0	Front to Rear	Daylight	Dry	Following Too Close
14	0	0	Front to Rear	Daylight	Dry	Driver Inattention
15	0	0	Front to Rear	Daylight	Wet	Following Too Close
16	0	0	Front to Rear	Daylight	Dry	Following Too Close
17	0	0	Front to Rear	Daylight	Wet	Failed to Yield Right of Way
18	0	0	Front to Rear	Daylight	Dry	Driver Inattention
19	0	0	Front to Rear	Daylight	Wet	Following Too Close
20	0	0	Front to Rear	Daylight	Dry	Driver Inattention
21	0	0	Front to Rear	Daylight	Dry	Driver Inattention
22	0	0	Front to Rear	Daylight	Dry	Driver Inattention
23	0	0	Front to Rear	Dark-Lighted	Dry	Made Improper Turn
24	0	0	Front to Rear	Daylight	Dry	Disregard Traffic Signal
25	0	1	Front to Rear	Daylight	Wet	Failed to Yield Right of Way
26	0	0	Front to Rear	Dark-Lighted	Dry	Driver Inattention
27	0	0	Front to Rear	Daylight	Dry	Following Too Close

#	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC	
28	0	0	Front to Rear	Daylight	Dry	Made Improper Turn	
29	0	0	Front to Rear	Daylight	Dry	Driver Inattention	
30	0	0	Front to Rear	Daylight	Dry	Following Too Close	
31	0	0	Front to Rear	Daylight	Wet	Following Too Close	
32	0	0	Front to Rear	Daylight	Dry	Following Too Close	
33	0	0	Front to Rear	Daylight	Dry	Failed to Yield Right of Way	
34	0	1	Front to Rear	Daylight	Dry	Following Too Close	
35	0	1	Front to Rear	Daylight	Dry	Made Improper Turn	
36	0	0	Front to Rear	Daylight	Ice/Frost	Driver Inattention	
37	0	0	Front to Rear	Dark-Lighted	Dry	Driver Inattention	
38	0	1	Front to Rear	Dark-Lighted	Dry	Made Improper Turn	
39	0	0	Front to Rear	Daylight	Dry	Improper Lane Change	
40	0	0	Front to Rear	Dark-Lighted	Dry	Following Too Close	

#	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC
41	0	0	Front to Rear	Daylight	Wet	Failed to Yield Right of Way
42	0	0	Front to Rear	Daylight	Dry	Driver Inattention
43	0	0	Front to Rear	Daylight	Dry	Following Too Close
44	0	0	Front to Rear	Daylight	Dry	Made Improper Turn
45	0	0	Front to Rear	Daylight	Dry	Made Improper Turn
46	0	0	Front to Rear	Daylight	Dry	Driver Inattention
47	0	0	Front to Rear	Daylight	Dry	Failed to Yield Right of Way
48	0	0	Front to Rear	Daylight	Dry	Driver Inattention
49	0	1	Not a Collision Between Two Vehicles	Daylight	Dry	Failed to Yield Right of Way
50	0	0	Not a Collision Between Two Vehicles	Daylight	Dry	Mechanical Defects
51	0	1	Not a Collision Between Two Vehicles	Dark-Not Lighted	Dry	Failed to Yield Right of Way
52	0	1	Not a Collision Between Two Vehicles	Dark-Not Lighted	Dry	Failed to Yield Right of Way
53	0	0	Not a Collision Between Two Vehicles	Dark-Lighted	Wet	Following Too Close
54	0	0	Not a Collision Between Two Vehicles	Daylight	Dry	Improper Lane Change
55	0	1	Sideswipe, Opposite Direction	Dark-Lighted	Dry	Disregard Traffic Signal
56	0	0	Sideswipe, Same Direction	Daylight	Dry	Driver Inattention
57	0	0	Sideswipe, Same Direction	Daylight	Dry	Improper Lane Change
58	0	0	Sideswipe, Same Direction	Dark-Lighted	Dry	Failed to Yield Right of Way
59	0	0	Sideswipe, Same Direction	Daylight	Dry	Mechanical Defects
60	0	0	Sideswipe, Same Direction	Daylight	Dry	Failed to Yield Right of Way
61	0	0	Sideswipe, Same Direction	Dark-Lighted	Dry	Improper Lane Change
62	0	0	Sideswipe, Same Direction	Daylight	Dry	Made Improper Turn
63	0	0	Sideswipe, Same Direction	Dark-Lighted	Dry	Improper Lane Change
64	0	0	Sideswipe, Same Direction	Dark-Not Lighted	Dry	Improper Lane Change
65	0	0	Sideswipe, Same Direction	Dark-Lighted	Dry	Driver Inattention
66	0	0	Sideswipe, Same Direction	Daylight	Dry	Failed to Yield Right of Way
67	0	0	Sideswipe, Same Direction	Daylight	Dry	Improper Lane Change

#	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC	
68	0	1	Sideswipe, Same Direction	Dark-Not Lighted	Dry	Made Improper Turn	
69	0	0	Sideswipe, Same Direction	Daylight	Dry	Made Improper Turn	
70	0	0	Sideswipe, Same Direction	Dark-Lighted	Dry	Failed to Yield Right of Way	
71	0	0	Sideswipe, Same Direction	Daylight	Dry	Failed to Yield Right of Way	
72	0	0	Unknown	Daylight	Dry	Made Improper Turn	
73	0	0	Unknown	Dark-Lighted	Dry	Driver Inattention	
74	0	2	Angle	Dark-Lighted	Wet	Disregard Traffic Signal	
75	0	0	Angle	Daylight	Dry	Speeding	
76	0	1	Angle	Daylight	Dry	Disregard Traffic Signal	
77	0	0	Angle	Daylight	Dry	Disregard Traffic Signal	
78	0	2	Angle	Daylight	Dry	Failed to Yield Right of Way	
79	0	1	Angle	Dark-Lighted	Dry	Disregard Traffic Signal	
80	0	0	Angle	Daylight	Dry	Disregard Traffic Signal	

#	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC
81	0	2	Angle	Daylight	Dry	Disregard Traffic Signal
82	0	1	Angle	Dark-Lighted	Dry	Disregard Traffic Signal
83	0	0	Angle	Dark-Lighted	Dry	Disregard Traffic Signal
84	0	0	Angle	Dark-Not Lighted	Dry	Failed to Yield Right of Way
85	1	1	Angle	Daylight	Dry	Disregard Traffic Signal
86	0	0	Angle	Daylight	Dry	Disregard Traffic Signal
87	0	5	Angle	Dark-Lighted	Dry	Disregard Traffic Signal
88	0	0	Angle	Daylight	Wet	Disregard Traffic Signal
89	0	0	Angle	Daylight	Dry	Driver Inattention
90	0	0	Angle	Daylight	Dry	Mechanical Defects
91	0	0	Angle	Dark-Lighted	Dry	Made Improper Turn
92	0	0	Angle	Daylight	Wet	Disregard Traffic Signal
93	0	0	Angle	Daylight	Dry	Disregard Traffic Signal
94	0	0	Angle	Daylight	Dry	Disregard Traffic Signal
95	0	1	Angle	Daylight	Dry	Made Improper Turn
96	0	0	Angle	Daylight	Dry	Disregard Traffic Signal
97	0	4	Angle	Daylight	Dry	Disregard Traffic Signal
98	0	2	Angle	Daylight	Dry	Mechanical Defects
99	0	2	Angle	Dark-Lighted	Dry	Disregard Traffic Signal
100	0	0	Angle	Dark-Lighted	Dry	Speeding
101	0	0	Front to Front	Daylight	Dry	Made Improper Turn
102	0	0	Not a Collision Between Two Vehicles	Dark-Lighted	Dry	Speeding

#	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC
1	0	1	Not a Collision Between Two Vehicles	Daylight	Dry	Made Improper Turn
2	0	0	Front to Rear	Daylight	Dry	Following Too Close
3	0	1	Front to Rear	Daylight	Dry	Following Too Close
4	0	0	Angle	Daylight	Dry	Disregard Traffic Signal
5	0	0	Front to Rear	Daylight	Dry	Made Improper Turn
6	0	0	Angle	Daylight	Dry	Mechanical Defects
7	0	0	Front to Rear	Daylight	Dry	Following Too Close
8	0	0	Sideswipe, Same Direction	Daylight	Dry	Improper Passing
9	0	0	Front to Rear	Daylight	Dry	Driver Inattention
10	0	1	Not a Collision Between Two Vehicles	Dark-Lighted	Wet	Failed to Yield Right of Way1
11	0	0	Angle	Daylight	Wet	Disregard Traffic Signal
12	0	0	Front to Rear	Daylight	Dry	Following Too Close
13	0	2	Angle	Dark-Lighted	Wet	Failed to Yield Right of Way
14	0	0	Angle	Daylight	Dry	Disregard Traffic Signal
15	0	0	Sideswipe, Same Direction	Daylight	Dry	Made Improper Turn
16	0	0	Front to Rear	Daylight	Dry	Driver Inattention
17	0	2	Angle	Daylight	Dry	Failed to Yield Right of Way
18	0	0	Not a Collision Between Two Vehicles	Dark-Lighted	Dry	Driver Inattention
19	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way
20	0	0	Angle	Dark-Lighted	Dry	Following Too Close Following Too Close
21	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way
22	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way
23	0	0	Front to Rear	Daylight	Dry	Following Too Close
24	0	1	Angle	Daylight	Dry	Failed to Yield Right of Way
25	0	0	Sideswipe, Same Direction	Daylight	Wet	Failed to Yield Right of Way
26	0	0	Front to Rear	Dark-Lighted	Dry	Following Too Close
27	0	1	Front to Front	Daylight	Wet	Failed to Yield Right of Way
28	0	0	Angle	Dark-Lighted	Wet	Failed to Yield Right of Way
29	0	0	Angle	Daylight	Wet	Failed to Yield Right of Way
30	0	0	Front to Rear	Daylight	Wet	Made Improper Turn
31	0	0	Front to Rear	Daylight	Dry	Following Too Close
32	0	0	Sideswipe, Same Direction	Daylight	Dry	Driver Inattention
33	0	0	Angle	Daylight	Wet	Made Improper Turn
34	0	0	Sideswipe, Same Direction	Dark-Lighted	Dry	Made Improper Turn
35	0	1	Angle	Daylight	Dry	1Failed to Yield Right of Way

#	Fatality	Injury	Manner of Impact	Lighting Condition	sc	PCC
36	0	1	Angle	Daylight	Wet	Mechanical Defects
37	0	0	Front to Rear	Dark-Not Lighted	Dry	Driver Inattention
38	0	0	Front to Rear	Daylight	Dry	Improper Passing
39	0	0	Sideswipe, Same Direction	Daylight	Dry	Failed to Yield Right of Way
40	0	2	Front to Front	Daylight	Dry	Made Improper Turn
41	0	0	Front to Rear	Daylight	Dry	Driver Inattention
42	0	3	Angle	Daylight	Wet	Failed to Yield Right of Way
43	0	2	Front to Front	Dark-Lighted	Dry	Failed to Yield Right of Way
44	0	0	Sideswipe, Same Direction	Daylight	Dry	Failed to Yield Right of Way
45	0	1	Angle	Dark-Lighted	Wet	Failed to Yield Right of Way
46	0	1	Front to Rear	Daylight	Dry	Driver Inattention
47	0	1	Angle	Daylight	Wet	Failed to Yield Right of Way
48	0	0	Angle	Daylight	Dry	Driving Under the Influence
49	0	0	Front to Rear	Daylight	Dry	Driver Inattention
50	0	0	Front to Rear	Dark-Lighted	Dry	Made Improper Turn
51	0	0	Sideswipe, Same Direction	Daylight	Dry	Driver Inattention
52	0	0	Sideswipe, Same Direction	Dark-Lighted	Dry	Made Improper Turn
53	0	0	Angle	Daylight	Ice/Frost	1Failed to Yield Right of Way
54	0	0	Front to Rear	Daylight	Dry	Driver Inattention
55	0	0	Front to Rear	Daylight	Dry	Made Improper Turn
56	0	0	Angle	Daylight	Dry	Made Improper Turn
57	0	0	Angle	Dark-Lighted	Dry	Made Improper Turn
58	0	0	Sideswipe, Same Direction	Dark-Lighted	Wet	Improper Lane Change
59	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way
60	0	2	Front to Front	Dark-Lighted	Dry	Failed to Yield Right of Way
61	0	0	Angle	Daylight	Dry	Failed to Yield Right of Way
62	0	0	Sideswipe, Same Direction	Dark-Lighted	Dry	Driver Inattention
63	0	1	Not a Collision Between Two Vehicles	Daylight	Dry	Failed to Yield Right of Way1
64	0	0	Angle	Dark-Lighted	Dry	Failed to Yield Right of Way
65	0	2	Angle	Daylight	Dry	Failed to Yield Right of Way
66	0	0	Sideswipe, Opposite Direction	Daylight	Dry	Failed to Yield Right of Way



**After Study** 

	Lind Avenue & Memorial Drive Reported Crashes, October 2019 - February 2020											
#	Manner of Impact Primary Contributing Circumstance Lighting Condition Weather Surface Fat											
1	Front to rear Driver inattention, distraction, or fatigue Dark-Lighted Clear Dry 0 0											

	Bizzare Drive & Memorial Drive Reported Crashes, October 2019 - February 2020									
#	# Manner of Impact Primary Contributing Circumstance Lighting Condition Weather Surface Fat						lnj			
1	Front to rear	Unknown (hit and run)	Dark-Lighted	Rain	Wet	0	0			
2	Front to rear	Driver inattention, distraction, or fatigue	Daylight	Clear	Dry	0	0			

	Karlyn Drine (east) & Memorial Drive Reported Crashes, October 2019 - February 2020									
# Manner of Impact Primary Contributing Circumstance Lighting Condition Weather Surface Fat						Fat	Inj			
1	Angle	Failed to yield right of way	Daylight	Clear	Dry	1	8			
2	Not a collision between two vehicles (Fixed Object - Tree)	Other environmental circumstances - weather, glare	Dark-Lighted	Rain	Wet	0	0			

	Karlyn Drive (west) & Memorial Drive Reported Crashes, October 2019 - February 2020									
#	# Manner of Impact Primary Contributing Circumstance Lighting Condition Weather Surface Fat II									
1	Front to rear	Driver inattention, distraction, or fatigue	Dark-Lighted	Unknown	Wet	0	0			

#### US 13 & Memorial Drive Reported Crashes, October 2019 - February 2020

#	Manner of Impact	Primary Contributing Circumstance	Lighting Condition	Weather	Surface	Fat	lnj
1	Angle	Disregard Traffic Signal	Dark-Lighted	Clear	Dry	0	1
2	Front to rear	Driver inattention, distraction, or fatigue	Daylight	Cloudy	Wet	0	0
3	Front to rear	Driver inattention, distraction, or fatigue	Daylight	Clear	Dry	0	0
4	Front to rear	Following too close	Daylight	Cloudy	Dry	0	0
5	Front to rear	Driver inattention, distraction, or fatigue	Daylight	Clear	Dry	0	0
6	Front to rear	Driver inattention, distraction, or fatigue	Daylight	Clear	Dry	0	0
7	Front to rear	Driver inattention, distraction, or fatigue	Dark-Lighted	Clear	Dry	0	0
8	Front to rear	Following too close	Daylight	Clear	Dry	0	0
9	Front to rear	Driver inattention, distraction, or fatigue	Daylight	Clear	Dry	0	0
10	Front to rear	Mechanical defects	Daylight	Rain	Wet	0	0
11	Angle	Driving in a careless or reckless manner	Daylight	Rain	Wet	0	3

#### Route 9 & Memorial Drive Reported Crashes, October 2019 - February 2020

#	Manner of Impact	Primary Contributing Circumstance	Lighting Condition	Weather	Surface	Fat	lnj
1	Front to rear	Driving in a careless or reckless manner	Daylight	Rain	Wet	0	1
2	Angle	Disregard Traffic Signal	Dark-Lighted	Clear	Dry	0	0
3	Angle	Driver inattention, distraction, or fatigue	Dark-Lighted	Clear	Dry	0	1
4	Front to rear	Driver inattention, distraction, or fatigue	Daylight	Clear	Dry	0	0
5	Not a collision between two vehicles (pedestrian)	Driving in a careless or reckless manner	Dark-Not Lighted	Unknown	Unknown	0	1
6	Angle	Failed to yield right of way	Daylight	Rain	Wet	0	0



APPENDIX E
Gap Study Results



-		_	W 7 - 17	
Ga	2		-	ets.
Ga		•	ш	uv

Project: Memorial Drive. Gap Study
Major Road:

Major Road: Memorial DV

Peak Hour/Date of Field Visit: 116,2020

W	10	-	+1	h	0	į,
	-	a	54	L.R	-	

				N	umber of Gaps					
	Period 1		Period 2		Period 3		Period 4			
Gap	From: 7:15 7:00	MA	From: - 30 741	MAC	From: 7:45 7:38 AM		From: 8: 007:45 AM			
Size	To: 7:30 7:15		To: 7.45 7430		To: 8:00 7:45		To:8:15 8:00			
(Sec)	Tally	Total	Tally	Total	Tally	Total	Tally	Total		
1			11	2						
2	111111	6	minimi	12	HIMINI	12	111111111111111111111111111111111111111	11		
3	WHITHIN	12	1111111111	10	1/11/11/11/11	14	HIIIII	8		
4	111	3	MIMILE	7	MILLIAMATA	14	11111	5		
5	11111	5	111	3	1111111111	10	11111111	9		
6	11111	5	1111	4	1111	4	11111111	9		
7	minim	9	111111	R	minni	8	11111	5		
8	11	2		1	1	1	1/1/11	6		
9	1111	4	11	2	11	2	11111	5		
10	1111	4	111	3	1111111	7	MI	3		
11			11	2	1111	+	1	1		
12	1	1		1	111	3	1111	4		
13	11	2	111	3	111	3	11	2		
14	11	2		3	17	2	111111	6		
15	111	3		)	IH	3	11,	2		
16			1	1	1.		11	2		
17	1	1	10	1	11	3	11	2		
18			M	3		1				
19			11							
20			11	5			1	1		
21		1000	1	4	1	1	11	5		
22		200			1	1				
23					1	1	1			
24		1			1	1		,		
25					1	1				
26		The same of				7 33		STEEL		
27		(Control of		1000	DOG FOR	300				
28					The second					
29		Start and		-50						
30						THE REAL PROPERTY.				
31			1	)			CARL CONTRACTOR			
32		The second second			3/	1	35			

33 1 42 1

45 32



**Gap Study** 

Project: Mimorial
Major Road: Mam

Peak Hour/Date of Field Visit: 116.2020

Weather:

Gap	Period 1				lumber of Gaps					
Gap			Period 2		Period 3		Period 4			
	From:3:00pm7100	MA	From: 3:15 7:15	MA	From: 3 30 7-30	AN	From: 3: 45 7:45 AM			
Size	To:3:15 7:15 AM		To: 3 30 7:30	MAC	To: 3:45 7:45	AM	To: 4:00 ~~ 8:00	AM		
(Sec)	Tally	Total	Tally	Total	Tally	Total	Tally	Total		
1					1					
2	11	1000			1111		9141			
3	11/11/11/11		ull		1111/1/11/11	3	WHAT I H			
4	MIMMIN		min		111111111111111111111111111111111111111		111111111111111111111111111111111111111			
5	1111111		HHIHH		111111111		minimin			
6	111111		MUMMI		11111		111111111			
7	1/11/1	0 -	11111		111111111		11/11/11			
	innin	8	HH	4	numu	9	Mil	4		
9	11	2	11111	6	111111111111	51	11	2		
10	1/1	3	IU	3	111	3	1			
11	111	3	11	2	111	3				
	HILLING	10	111	3	17/11	5	111	3		
13	1	7	11	3	1111	5	11	5		
14			1	1	1	1	1	1		
15	111	3			11	2	11	2		
16	11	2			11	2		1		
17	1	1	111	3	•					
18	11	5	1111	4	1	1		1		
19		7		1		1	1	1		
20		1	11	5		1	1	1		
21	111	3				1	11	5		
22		1	11	2		1	111	3		
23						3	1	1		
24				1						
25	1	1				1	2/5	Market Barrier		
26	1	1		1						
27			1	1						
28		1								
29				1			1	1		
30										
31				1200		4.6.8				
32			The second second			4		1		

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Gap Study		
Project: Mam	prial Prive Gap Study	'u
Major Road:	Mamarial Drive	
Intersection:	Memorial Dr/Karlyn F	rive (east)
Peak Hour/Date	of Field Visit: 1/6, 7070	
Weather:		

		100		umber of Gaps				
1000	Period 1		Period 2		Period 3	3	Period 4	
Gap	From: 4:45 7:00 AM PA		From:5:00 7:15	AM	From:515 7:30 AMPM		From: 7:45 AMPM	
Size	To:5 00 7:45	AM pm	To: 5:15 7:30	LAM	To:5:30 7:45	-AM pm	To: 5' 45 8:00	MPM
(Sec)	Tally	Total	Tally	Total	Tally	Total	Tally	Total
1						3		
2	111111111111		IIIIIIIII		mill		milli	
3	11 million		11/11/11/11		MIMM		MILLI	
4	1/11/11/11/11		MILLIMIN		munum		11111111	
5	HIM MILLI		MINIMA		munnin		minnin	
6	61111111		MILIMIN		ununn,		mmmm	
7	*111		1/1/1/1/1/1		Manney		MMMM	
8	imm	9	111	3	11111	5	1111111	7
9	11	7	11111	5	111	3	111)	4
10	11	2	1111	5	11111	5		
11	111	3	111	3	1111	4	11111	5
12	(1)	3	() []	4	111	3	111	3
13	[[]	3		1			111	3
14	1/1/	4			11	2	1111111	7
15				1				
16	1	1	1	1			11	2
17	1	1	1	1	11	2		
18				1				
19		100	11	5				
20		1	H	5	1111	4		The same
21	1	1		-	1	1	111	3
22		1	11	2	1	1	1	
23		1		72	1			
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25		1	1	1			11	7
26		1		George .	11	5		
27				1	A STATE OF THE STA			100
28		-	Parl			Marie Land	1	
29		1					1	
30	Will be the second					Sec.		
31	11	7					10.000	
32				441	The same of	1000		
-		1	3<				3/	