PROJECT NOTES

- 1. <u>LOCATION</u>
 INSTALLATION OF NEW RIGID FRAME CULVERT CARRYING US301 OVER
 WILDLIFE CROSSING AND CHANNEL IN NEW CASTLE COUNTY, DELAWARE.
- 2. <u>ELEVATIONS</u>
 VERTICAL DATUM IS REFERENCED TO NAVD 1988.
- 3. <u>DESIGN CRITERIA</u>
 2007 AASHTO LRFD DESIGN SPECIFICATIONS, INCLUDING 2008 AND 2009
 INTERIMS, AND SUPPLEMENTED BY THE DELAWARE DEPARTMENT OF
 TRANSPORTATION 2005 BRIDGE DESIGN MANUAL, INCLUDING REVISIONS
 THROUGH 2009.
- 4. LOADING
 LIVE LOAD: AASHTO HL-93 AND DELAWARE LEGAL LOADS.
 FUTURE OVERLAY = 25 P.S.F.
 FILL SOIL = 120 P.C.F.
- FOUNDATIONS
 FOUNDATION DESIGN AND QUANTITIES ARE BASED ON A STUDY
 OF THE SUBSOIL BORINGS MADE AT THE SITE. THE BORING
 INFORMATION SHOWN ON THE SOIL BORING SHEET IS BASED ON
 LIMITED INVESTIGATIONS AND IS IN NO WAY WARRANTED TO BE
 INDICATIVE OF ACTUAL CONDITIONS THAT MAY BE ENCOUNTERED
 DURING CONSTRUCTION. SEE SECTION 102.05 OF THE STANDARD
 SPECIFICATIONS FOR MORE DETAIL. THIS BORING INFORMATION, THE
 ACCURACY OF WHICH THE STATE DOES NOT GUARANTEE, IS PRESENTED
 ON THESE PLANS TO THE CONTRACTOR FOR HIS INFORMATION ONLY.

THE CRITICAL APPLIED PRESSURE FOR THE CONTROLLING LOAD CASES FOR THE PEDESTALS AND WINGWALLS ARE 6.66 KSF AND 5.18 KSF RESPECTIVELY WHICH IS LESS THAN THE BEARING RESISTANCES OF 7.09 KSF AND 5.70 KSF RESPECTIVELY.

6. <u>RIGID FRAME</u>
FACTORED BEARING RESISTANCE (STR 1) = 7.09 ksf
MAXIMUM BEARING PRESSURE (STR 1) = 6.66 ksf
PHI (BEARING CAPACITY) = 0.45

WINGWALL:

FACTORED BEARING RESISTANCE (STR 1) = 5.70 ksf

MAXIMUM BEARING PRESSURE (STR 1) = 5.18 ksf

PHI (BEARING CAPACITY) = 0.45

MAXIMUM HORIZONTAL FORCE AT FULL HEIGHT (STR 1) = 19.7k

FACTORED SLIDING RESISTANCE AT FULL HEIGHT (STR 1) = 21.6k

PHI (SLIDING) = 0.8

FOOTINGS MAY BE ORDERED BY THE ENGINEER TO BE AT ANY ELEVATION OR OF ANY DIMENSION NECESSARY TO PROVIDE PROPER FOUNDATION.

7. CONCRETE

ALL CONCRETE PROPERTIES SHALL BE IN ACCORDANCE WITH DIVISION 812 OF THE DELAWARE DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS.

CLASS A - HEADWALLS, PEDESTAL WALLS, AND WINGWALLS (f'c = 4,500 PSI).

CLASS B - WINGWALL FOOTING AND PEDESTAL WALL FOOTING (f'c = 3,000 PSI).

PRECAST RIGID FRAME CULVERT CONCRETE (f'c = 5000 PSI).

ALL EXPOSED EDGES SHALL BE CHAMFERED 3/4" UNLESS NOTED OTHERWISE.

8. <u>REINFORCING STEEL</u>

ALL REINFORCING STEEL SHALL BE AASHTO M31 (ASTM A615), GRADE 60 AND UNLESS SPECIFIED OTHERWISE ON THE PLANS SHALL BE PROTECTED WITH FUSION BONDED EPOXY, CONFORMING TO AASHTO M284 (ASTM D3963) AND DENOTED WITH A PREFIX 'E' IN THE BAR MARKS.

MINIMUM CONCRETE COVER FOR REINFORCING STEEL SHALL BE:

PRECAST CULVERT: 2"
FOUNDATION ELEMENTS: 3"
OTHER ELEMENTS: AS NOTED

- 9. POST-TENSIONING TENDONS AND ANCHOR PLATE
 LOW RELAXATION POLY PROPYLENE-SHEATHED PRESTRESSING STRAND WITH
 CORROSION INHIBITOR SHALL CONFORM TO AASHTO M203 (ASTM A416),
 GRADE 270. STRUCTURAL STEEL FOR ANCHOR PLATES SHALL CONFORM TO
 AASHTO M270 (ASTM A709), GRADE 36 AND SHALL BE FUSION BONDED
 EPOXY COATED. THREADS ON TIE RODS SHALL BE CUT TO COARSE SERIES
 2A. PAYMENT SHALL BE INCIDENTAL TO ITEM 602522 PRECAST CONCRETE
 CULVERT.
- 10. THREADED INSERTS
 PROVIDE THREADED INSERTS AS INDICATED. THREADED INSERTS
 ARE TO BE INCORPORATED INTO THE PRECAST BOX SEGMENTS BY
 THE FABRICATOR.

11. CONSTRUCTION

FOR CONSTRUCTION SEQUENCE, SEE DRAWING DT-04.

POST TENSION PRECAST CULVERT SEGMENTS IN ACCORDANCE WITH DETAILS AND NOTES ON DRAWINGS DT-06 AND DT-08.

POST TENSION THE PRECAST SEGMENTS PRIOR TO BACKFILLING AND PRIOR TO ALLOWING TRAFFIC ON THE CULVERT.

TREAT CULVERT SURFACES WITH SILANE SEALER PRIOR TO BACKFILLING.

APPLY WATERPROOFING TO THE JOINTS OF THE PRECAST CULVERT AS INDICATED.

USE EPOXY BONDING COMPOUND BETWEEN PRECAST AND CAST-IN-PLACE ELEMENTS. EPOXY BONDING COMPOUND IS TO BE TYPE 2, GRADE 2 AS DESCRIBED IN ASTM-C881.

DO NOT EXCEED A 2'-O" DIFFERENCE IN FILL ELEVATIONS ON THE SIDES OF THE CULVERT DURING PLACEMENT OF THE BACKFILL.

DO NOT ALLOW WHEELS OF ROLLERS TO COME CLOSER THAN 1'-O" TO FACE OF STRUCTURE DURING COMPACTION OF THE BACKFILL.

PROVIDE MATERIAL AND PERFORM WORK IN ACCORDANCE TO THE DELAWARE DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS AND CONSTRUCTION DETAILS AND CONTRACT SPECIAL PROVISIONS.

- 2. <u>CONSTRUCTION JOINTS</u>
 KEYED CONSTRUCTION JOINTS SHALL BE 2"x4" OR AS NOTED.
 ALL EXPOSED CONSTRUCTION JOINT EDGES SHALL HAVE 3/4" V-NOTCH.
- . STABILIZING STRUCTURAL EXCAVATIONS
 IN LIEU OF A 2: 1 SLOPE THE CONTRACTOR MAY USE SHORING
 FOR ANY EXCAVATIONS EXCEEDING 5 FEET IN HEIGHT. THE
 COST OF SHORING SHALL BE INCIDENTAL TO ITEM 207000
 EXCAVATION AND BACKFILL FOR STRUCTURES.

THE CONTRACTOR IS RESPONSIBLE FOR THE STABILITY OF ALL EXCAVATED SLOPES.

DIRECT ALL SURFACE RUNOFF AWAY FROM EXCAVATION USING CURBING OR A

BARRIER ALONG TOP OF SLOPE. IF REQUIRED, COVER THE EXCAVATED SLOPES WITH
PLASTIC TO PROTECT AGAINST INFILTRATION.

PERFORM EXCAVATION IN ACCORDANCE WITH O.S.H.A. REQUIREMENTS.

14. UTILITIES

BEFORE BEGINNING WORK, THE CONTRACTOR SHALL GIVE NOTIFICATION BY TELEPHONE BY CALLING "MISS UTILITY" AT 1-800-282-8555 A MINIMUM OF 48 HOURS PRIOR TO START OF WORK. VERIFY AND LOCATE ALL UTILITIES PRIOR TO STARTING WORK.

COORDINATE THE REQUIREMENTS FOR PROTECTION OF ANY UTILITY WITH THE UTILITY OWNER PRIOR TO STARTING WORK.

CONDUCT OPERATIONS IN A MANNER WHICH ENSURES THAT THE UTILITIES WILL NOT BE DISTURBED OR ENDANGERED. ANY DAMAGE INCURRED TO THESE UTILITIES OR ANY OTHER UTILITIES, SHOWN OR NOT SHOWN ON THE PLANS, DUE TO THE CONTRACTOR'S OPERATIONS SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE TO THE SATISFACTION OF THE APPROPRIATE UTILITY COMPANY. THE DEPARTMENT DOES NOT ASSUME RESPONSIBILITY FOR REIMBURSEMENT, PARTICIPATION IN DESIGN AND/OR REVISIONS, OR LIABILITY FOR ACCURACY OF TYPE, SIZE AND LOCATION OF ANY UTILITY.

THE CONTRACTOR IS RESPONSIBLE FOR TEMPORARILY SUPPORTING, PROTECTING, OR RELOCATING ANY UTILITIES DURING CONSTRUCTION. WHERE NECESSARY, THE COST FOR THIS WORK WILL BE INCIDENTAL TO THE CONTRACT.

- 15. LOAD RATINGS
 THE LOAD AND RESISTANCE FACTOR METHOD WAS USED TO LOAD RATE THIS STRUCTURE SEE LOAD RATING TABLE, THIS SHEET.
- 6. <u>DEL NO. 3/57 STONÉ MIX</u>

 USE A MIX OF 50% DEL NO.3 STONE, ITEM 302011 AND 50% DEL NO. 57 STONE, ITEM 302012. PAYMENT FOR MIXING AND PLACING DEL NO. 3/57 STONE MIX SHALL
- 17. PRECAST ELEMENT NOTES.
 SEE DRAWING DT-08.

BE INCIDENTAL TO EACH ITEM.

INDEX OF DRAWINGS					
SHEET NO.	DRAWING NO.	TITLE			
581	BR1-444DT-01	PROJECT NOTES & QUANTITIES			
582	BR1-444DT-02	GENERAL PLAN AND ELEVATION			
583	BR1-444DT-03	CULVERT END EL. VIEWS AND TYPICAL SECTION			
584	BR1-444DT-04	STAKE-OUT PLAN			
585	BR1-444DT-05	CAST IN PLACE WALL FOUNDATION PLAN			
586	BR1-444DT-06	CAST IN PLACE WALL AND SEGMENT PLAN AND ELEVATIONS			
587	BR1-444DT-07	WINGWALL FOUNDATION PLAN			
588	BR1-444DT-08	PRECAST RIGID FRAME CULVERT DETAILS			
589	BR1-444DT-09	HEADWALL DETAILS			
590	BR1-444DT-10	WINGWALLS A & B DETAILS			
591	BR1-444DT-11	WINGWALLS C & D DETAILS			
592	BR1-444DT-12	REINFORCEMENT BAR SCHEDULE 1			
593	BR1-444DT-13	REINFORCEMENT BAR SCHEDULE 2			
594	BR1-444DT-14	SOIL BORING LOGS			

QUANTITIES						
ITEM NO.	ITEM TITLE	UNIT	QUANTITY			
207000	EXCAVATION AND BACKFILL FOR STRUCTURES	C. Y.	2413			
302011	DEL NO. 3 STONE	TON	155			
302012	DEL NO. 57 STONE	TON	337			
602001	PORTLAND CEMENT CONCRETE MASONRY, CLASS A	C. Y.	283			
602002	PORTLAND CEMENT CONCRETE MASONRY, CLASS B	C. Y.	458			
602522	PRECAST CONCRETE CULVERT	L.F.	160.5			
604000	BAR REINFORCEMENT, EPOXY COATED	LB	82600			
608000	COARSE AGGREGATE FOR FOUNDATION STABILIZATION AND SUBFOUNDATION BACKFILL	TON	835			
712021	RIPRAP, R-5	TON	985			
712531	CHANNEL BED FILL	C. Y.	69			
715001	PERFORATED PIPE UNDERDRAIN, 6"	L.F.	463			

LEGEND

SHEET NOS. REFERENCE STRUCTURE PLAN SHEETS UNLESS NOTED OTHERWISE

ARCH.	=	ARCHITECTURAL	F. F.	=	FRONT FACE
B <u>.</u>	=	BASELINE	GAL V.	=	GALVANIZED
B. F. E.	=	BOT <mark>TO</mark> M OF	MAX.	=	MAXIMUM
		FOOTING ELEVATION	MIN.	=	MINIMUM
вот.	≥	BOTTOM	NOM.	=	NOMINAL
Q.	=	CENTERLINE	N. T. S.	=	NOT TO SCALE
C. I. P.	=	CAST-IN-PLACE	P.G.L.	=	PROPOSED GRADE LINE
CLR.	=	CLEAR	R.C.	=	REINFORCED CONCRETE
CONC.	=	CONCRETE	R.F.	=	REAR FACE
CONSTR	. =	CONSTRUCTION	S.E.	=	SUPERELEVATION
CVR.	=	COVER	SHLDR.	=	SHOULDER
DIA.	=	DIAMETER	STA.	=	STATION
E.F.	=	EACH FACE	THICK.	=	THICKNESS
EL.	=	ELEVATION	THRD.	=	THREADED
E.S.	=	EQUAL SPACING	TYP.	=	TYPICAL
			W. P.	=	WORK POINT

LOAD RATING SUMMARY						
DESIGN VEHICLE	RATING FACTOR	RATING WEIGHT (TON)	CONTROLLING MEMBER	CONTROLLING POINT	LOAD EFFECT	
HL-93 T <mark>RUC</mark> K (INVENTORY)	1.64	N/A	TOP SLAB	LEFT END	SHEAR	
HL-93 TANDEM (INVENTORY)	1.50	N/A	TOP SLAB	LEFT END	SHEAR	
HL-93 TRUCK (OPERATING)	2.12	N/A	TOP SLAB	LEFT END	SHEAR	
HL-93 TANDEM (OPERATING)	1 . 95	N/A	TOP SLAB	LEFT END	SHEAR	
DE S220 (LEGAL)	2. 95	<i>59.07</i>	TOP SLAB	LEFT END	SHEAR	
DE S335 (LEGAL)	1.62	56 . 82	TOP SLAB	LEFT END	SHEAR	
DE S437 (LEGAL)	1.62	59.48	TOP SLAB	LEFT END	SHEAR	
DE T330 (LEGAL)	2.84	<i>85.17</i>	TOP SLAB	LEFT END	SHEAR	
DE T435 (LEGAL)	2.24	<i>78.</i> 42	TOP SLAB	LEFT END	SHEAR	
DE T540 (LEGAL)	2.19	87.50	TOP SLAB	LEFT END	SHEAR	
NOTE: LOAD RATING INCLUDES FUTURE WEARING SURFACE AS NOTED IN THE PLANS.						

BR1-444DT-01

DELAWARE
DEPARTMENT OF TRANSPORTATION

ADDENDUMS / REVISIONS

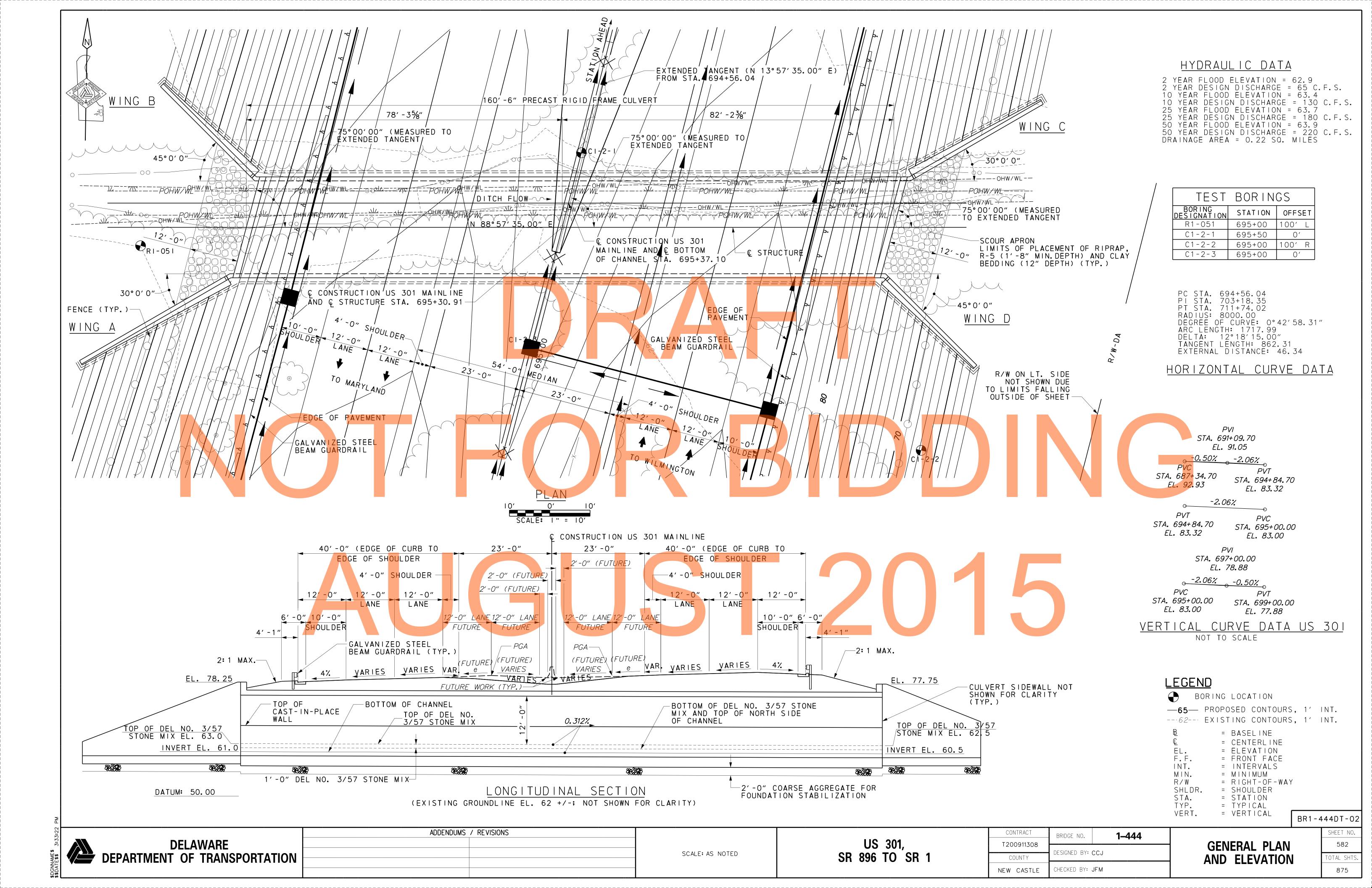
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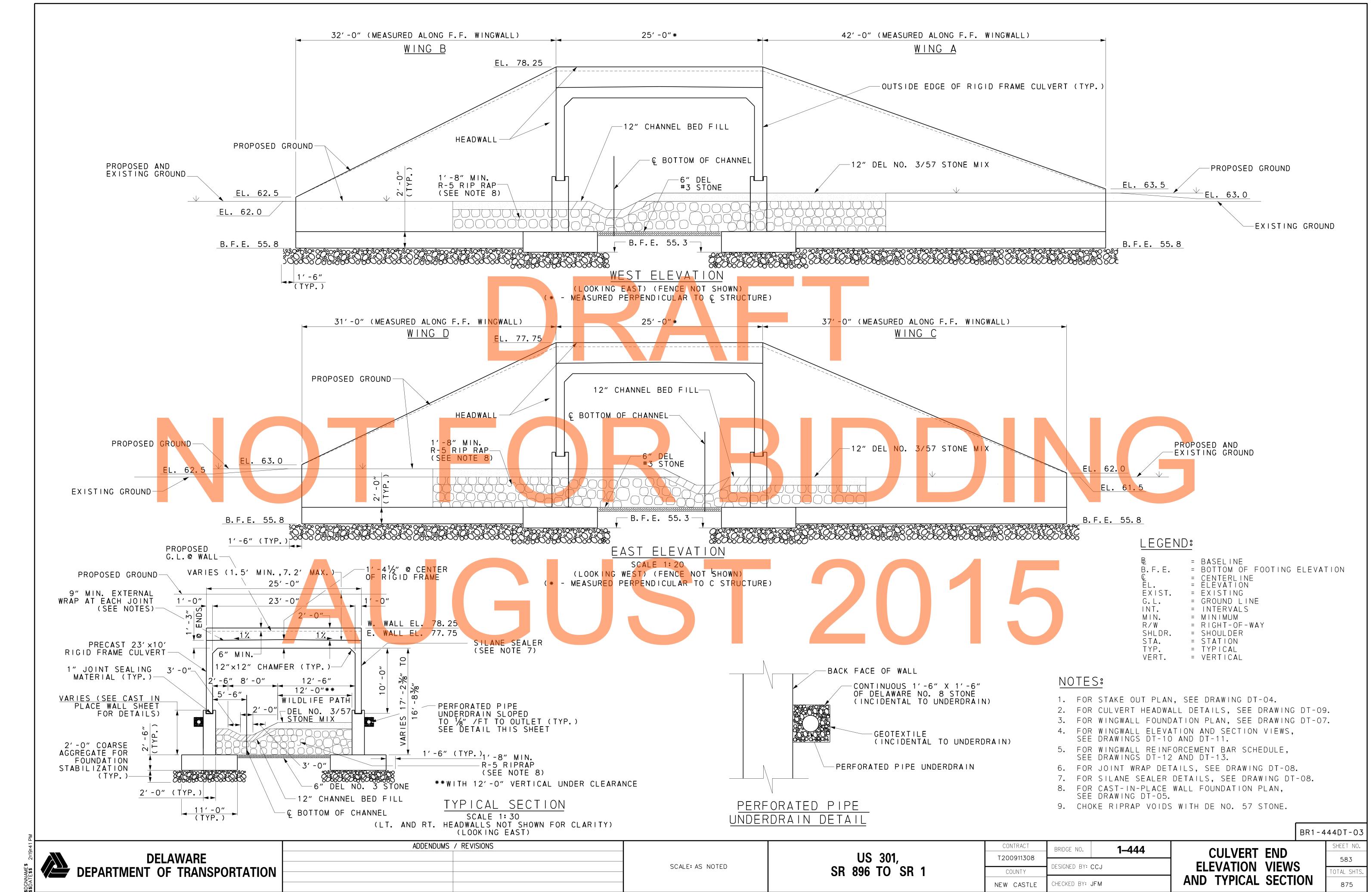
US 301, SR 896 TO SR 1

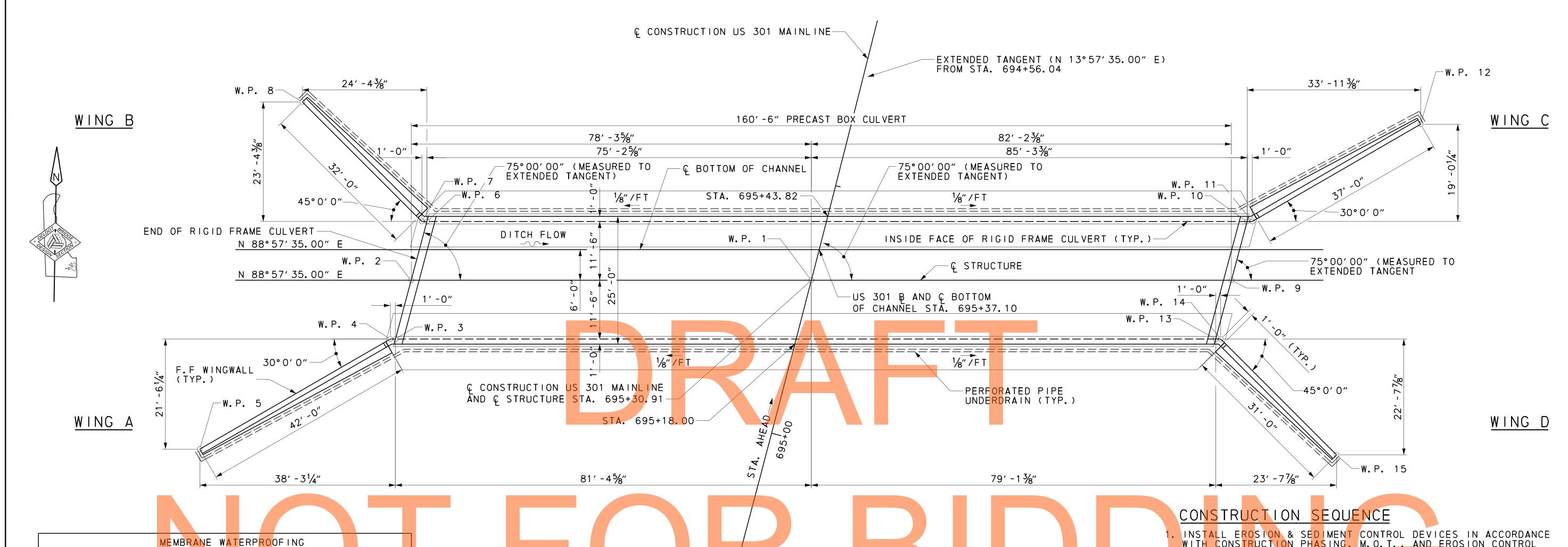
CONTRACT	BRIDGE NO.	1–444	
T200911308		1 777	
COUNTY	DESIGNED BY:	CCJ	
NEW CASTLE	CHECKED BY:	JFM	

PROJECT NOTES AND QUANTITIES

SHEET NO.
581
TOTAL SHTS.
875







		ERP <mark>RO</mark> OFING , DT-10 AND D1	⁻ – 1 ₁)
		PREFORMED MEM	MBRANE SHEETS
PROPERTY	TEST	RUBBERIZED ASPHALT TYPE	MODIFIED BITUMEN TYPE
TENSILE STRENGTH, bs/in (MIN.) (1)(3)	ASTM D 882 (2)	20	20
% ELONGATION AT BREAK, (MIN.) (3)(4)		25	25
PLIABILITY	ASTM D 146 (5)	NO CRACKS	NO CRACKS
THICKNESS, MILS (MIN.) (6)	ASTM D 1000	60	60
SOFTENING POINT, F (MIN.)	ASTM D 36	190	210
PERMEANCE, PERMS (MAX.)	ASTM D 96, METHOD B	0.1	0.1
PUNCTURE RESISTANCE Ibs. (MIN.)	ASTM E 154	40	40
NOTES:			

NOTES:

- (1) BREAKING FACTOR IN MACHINE DIRECTION.
- (2) METHOD A, 1-INCH WIDE STRIP WITH 4-INCH MINIMUM INITIAL SEPARATION AND 4-INCH GAGE LENGTH AT 2 INCHES PER MINUTE. AVERAGE 5 SAMPLES.
- (3) AT $73.4F \pm 3.6F$.
- (4) MACHINE DIRECTION.
- (5) 180-DEGREE BEND OVER A 1-INCH MANDREL AT-15F.
- (6) TOTAL THICKNESS OF PREFORMED MEMBRANE SHEET AND POLYETHYLENE FILM OR FABRIC REINFORCEMENT.

PC 694+56.04

W. P.	NORTHING	EASTING	B CONSTRUCTION US 301 MAINLINE		
			STATION	OFFSET	
1	549874.8819	578435. 3175	695+30.91	0.00'	
2	549873.4603	578357.0264	695+11.17	75.80′L	
3	549861.9062	578354.1543	694+99.15	75 . 87′ L	
4	549861.8881	578353. 1545	694+98.89	76.84′L	
5	549839.6940	578316.2761	694+68.21	107.39′L	
6	549885.0143	578359.8986	695+23.19	75.71′L	
7	549884.9962	578358.8987	695+22.93	76.68′L	
8	549907. 9321	578335.1144	695+39.91	105 . 13′ L	
9	549876.3742	578517.5000	695+51.24	79.62′R	
10	549887.9283	578520.3721	695+63.03	79.77′R	
11	549887.9464	578521.3719	695+63.27	80.74′R	
12	549907.5622	578553.9662	695+89.45	108.04′ R	
13	549864.8202	578514.6279	695+39.45	79 . 48′ R	
14	549864.8383	578515.6277	695+39.70	80.45′R	
15	549842.5965	578538.6922	695+23.60	108.05′R	

ADDENDUMS / REVISIONS

WORK POINT COORDINATES

INSTALL EROSION & SEDIMENT CONTROL DEVICES IN ACCORDANCE WITH CONSTRUCTION PHASING, M.O.T., AND EROSION CONTROL PLANS.

IMPLEMENT STREAM DIVERSION AS SPECIFIED IN CONSTRUCTION PHASING, M. O. T., AND EROSION CONTROL PLANS.

- 3. EXCAVATE AND DEWATER TO BOTTOM OF COARSE AGGREGATE FOR FOUNDATION STABILIZATION AS INDICATED IN STRUCTURE PLANS DRAWING DT-08 & IN ACCORDANCE WITH SECTION 207 OF THE STANDARD SPECIFICATIONS.
- 4. PLACE COARSE AGGREGATE FOR FOUNDATION STABILIZATION TO THE ELEVATIONS ON STRUCTURE PLAN DRAWING DT-03.
- 5. CAST PEDESTAL WALLS ACCORDING TO DRAWING DT-05.
- 6. CAST FOOTINGS FOR WINGWALLS ACCORDING TO DRAWING DT-07.
- PLACE RIGID FRAME CULVERT IN ACCORDANCE WITH PRECAST CONCRETÉ CULVERT SPECIAL PROVISIONS & MANUFACTURER'S SPECIFICATIONS. POST TENSION IN ACCORDANCE WITH DRAWINGS DT-06_AND DT-08 AND APPROVED ERECTION PLANS.
- CONSTRUCT HEADWALLS & WINGWALLS ACCORDING TO STRUCTURE PLAN DRAWINGS DT-09, DT-10, AND DT-11.
- 9. PLACE RIPRAP, CHANNEL BED FILL AND DEL NO. 3/DEL NO. 57 STONE MIX IN CULVERT AND AT WINGWALLS IN ACCORDANCE WITH STRUCTURE PLAN DRAWING DT-08 AND EROSION AND SEDIMENT CONSTRUCTION SEQUENCE. PRIOR TO PLACING THE DEL NO. 3/57 STONE MIX, FILL THE VOIDS IN THE RIPRAP ABOVE THE DEL NO. 3/57 STONE MIX WITH DEL NO. 3 STONES. THE VOIDS SHALL BE FILLED UNTIL THE PEAKS OF THE RIPRAP ARE BARELY VISIBLE. USE A MIX OF 50% DEL NO. 3 STONE AND 50% DEL NO. 57 STONE AT LOCATIONS WHERE "DEL NO. 3/57 STONE MIX" IS SPECIFIED. SEE PROJECT NOTES FOR PAYMENT.
- 10. WHEN BACKFILLING RIGID FRAME CULVERT DO NOT LET THE DIFFERENCES IN GROUND ELEVATION ON OTHER SIDE OF RIGID FRAME CULVERT EXCEED 2'.
- 11. RESTORE STREAM FLOW INTO RIGID FRAME CULVERT.

NOTES

1. FOR CAST IN PLACE PEDESTAL WALL FOOTING PLAN AND WORK POINTS, SEE DRAWING DT-05.

BR1-444DT-04

DELAWARE DEPARTMENT OF TRANSPORTATION

NOT TO SCALE

US 301, SR 896 TO SR 1

= BASELINE

= MAXIMUM

= MINIMUM

= STATION

= TYPICAL

= CENTERLINE = FRONT FACE

= WORK POINT

LEGEND

MAX.

MIN.

STA.

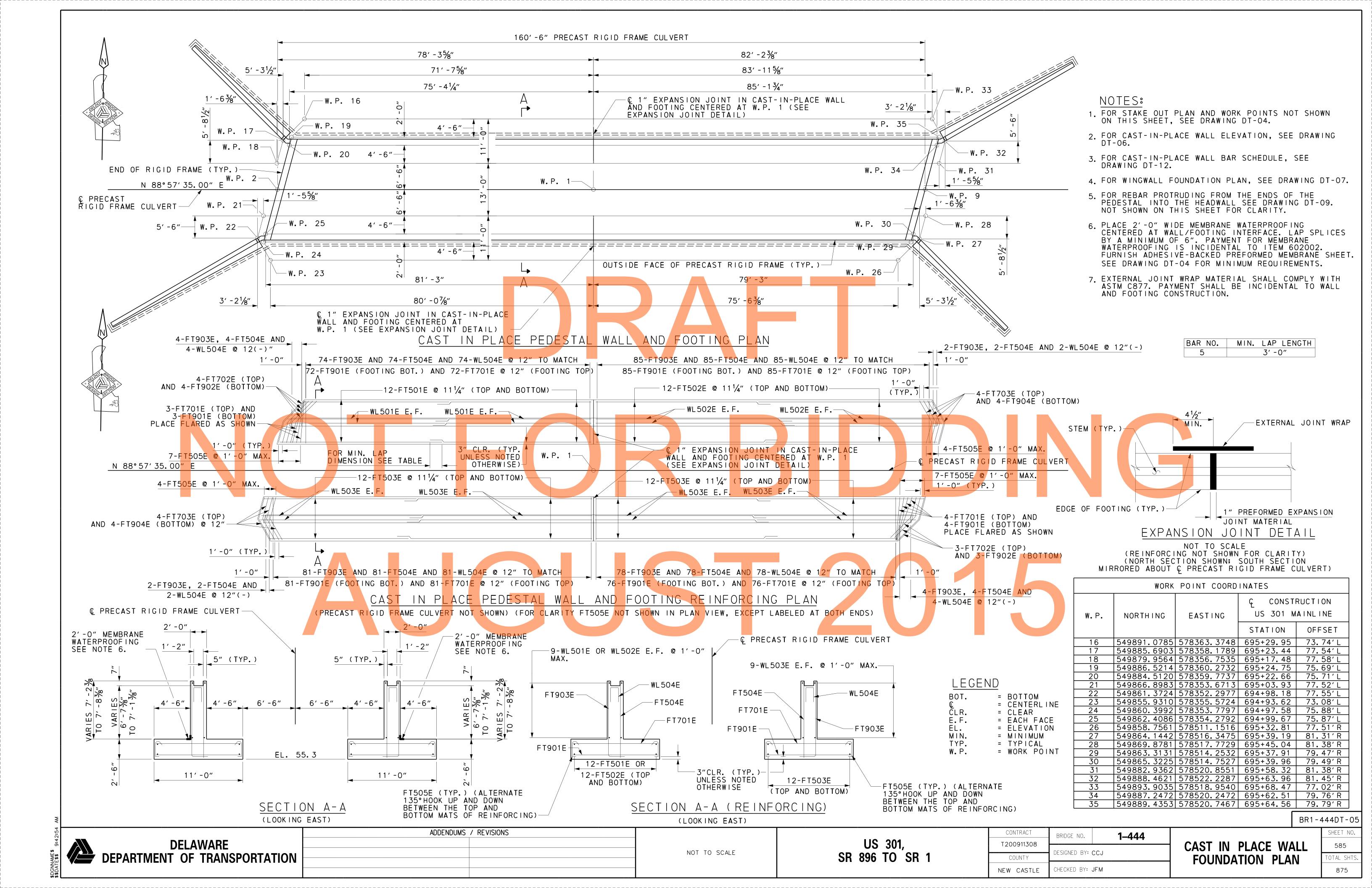
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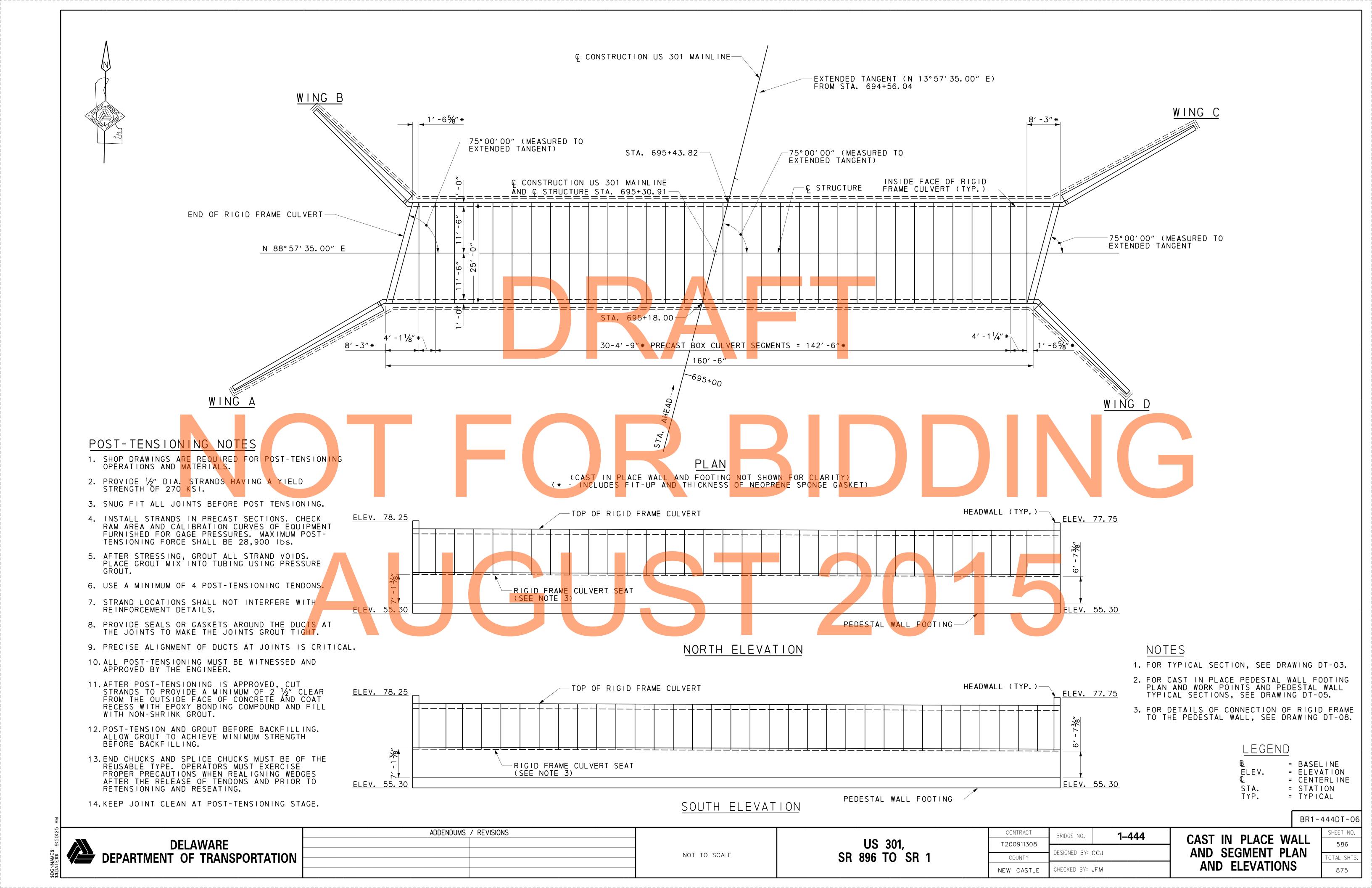
W. P.

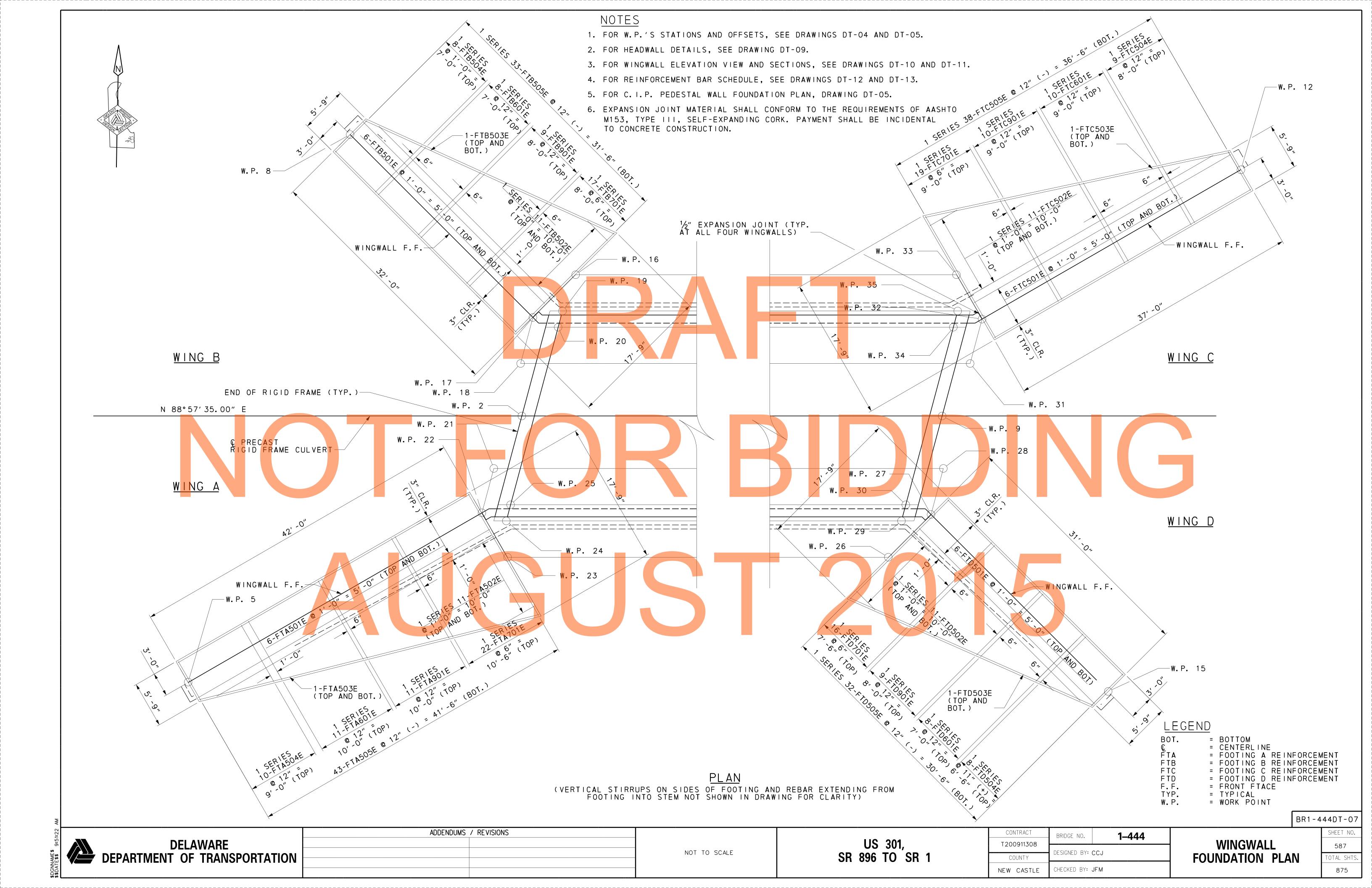
CONTRACT 1–444 BRIDGE NO. T200911308 DESIGNED BY: CCJ COUNTY CHECKED BY: JFM NEW CASTLE

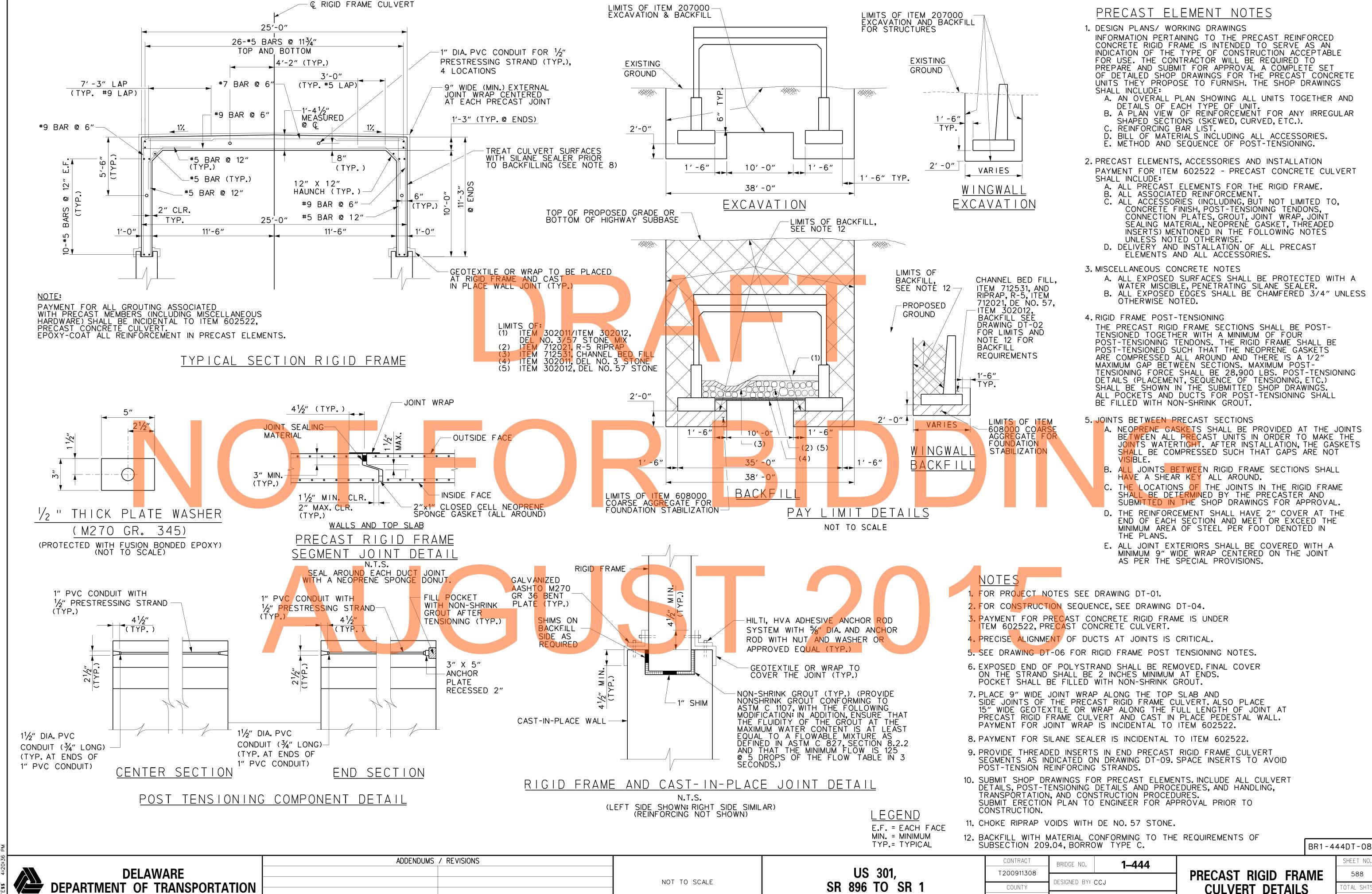
STAKE-OUT PLAN

OTAL SHTS 875









COUNTY CHECKED BY: JFM NEW CASTLE

CULVERT DETAILS

588 TAL SHTS 875

