

<u>GE</u>	<u>NERAL NOTES</u>			
1			SHEET NO. 523	DRAW 1-47
1.	AASUTO LEED RELEACE DESIGN SECTEMENTATIONS ATH EDITION 2007 INCLUDING	THE MINIMUM COMPRESSIVE STRENGTH FOR RRESTRESSED CONCRETE AT THE ACE OF	524 525	1-47
	2008 AND 2009 INTERIM REVISIONS, AND AS SUPPLEMENTED BY DELAWARE DEPARTMENT OF TRANSPORTATION BRIDGE DESIGN MANUAL, MAY 2005, INCLUDING LATEST REVISIONS.	28 DAYS SHALL BE f'c=8,000 PSI. THE MINIMUM COMPRESSIVE STRENGTH AT THE TRANSFER OF PRESTRESS SHALL BE f'c1=6,400 PSI.	526 527	1-47 1-47
	PROVIDE MATERIALS AND PERFORM WORK IN ACCORDANCE WITH THE DELAWARE DEPARTMENT	PRESTRESSED STEEL:	528	1-47
	OF TRANSPORTATION STANDARD SPECIFICATIONS AND CONSTRUCTION DETAILS, AASHTO/AWS D1.5M/D1.5 BRIDGE WELDING CODE, AND CONTRACT SPECIAL PROVISIONS.	PRETENSIONING STEEL FOR BEAMS SHALL CONSIST OF HIGH STRENGTH 7-WIRE LOW	530	1-47
	LIVE LOAD DISTRIBUTION TO BEAMS IS BASED UPON AASHTO DISTRUBUTION FACTORS.	RELAXATION STRANDS, WITH A NOMINAL 0.60 INCH DIAMETER, CONFORMING TO THE REQUIREMENTS OF AASHTO M2O3 (ASTM A416) GRADE 270. EACH 0.60 INCH DIAMETER	<u>531</u> 532	1-47
2.	LOADING	STRAND SHALL BE PRETENSIONED TO 43,942 LBS. (0.75 Fpu).	533	1-47
	UNIT WEIGHTS OF MATERIALS SHALL BE IN ACCORDANCE WITH THE DELAWARE BRIDGE DESIGN MANUAL.	AFTER ESTIMATED LOSSES OF 27,499 PSI, THE FINAL EFFECTIVE PRESTRESS FORCE PER STRAND IS 37,975 LBS.	535	1-47
	FUTURE OVERLAY ALLOWANCE SHALL BE 25 LBS/SQ FT.	6. SERVICEABILITY:	536 537	1-47
	STEEL BRIDGE DECK FORMS WHICH STAY IN PLACE (INCLUDING CONCRETE IN FORM CORRUGATIONS)	LIVE LOAD DEFLECTION SHALL BE LIMITED TO L/800.	538 539	1-47
	SHALL BE ID LEDIDU FI.	FOR REINFORCEMENT DISTRIBUTION REQUIREMENTS, CONSIDER CLASS 2 EXPOSURE CRITERIA	540	1-47
	OR TANDEM WITH DYNAMIC LOAD ALLOWANCE AND A LANE LOAD. RATINGS SHALL USE ALL DELAWARE		542	1-47
	RADDIED HAS BEEN DESIGNED FOR TEST LEVEL FOUR (TL-4)	KEYED CONSTRUCTION JOINTS SHALL BE 2" Y A" OP AS NOTED ALL EXPOSED CONSTRUCTION	543 544	1-47
	EATIGUE DESIGN IS BASED ON THE ENLIOWING:	JOINT EDGES SHALL HAVE A 3/4" V-NOTCH, UNLESS NOTED OTHERWISE.	545	1-47
	PRESTRESSED CONCRETE: ADTT 1,236 (2030 ONE-DIRECTIONAL).	8. STRUCTURAL BACKFILL:	547	1-47
	FOR THERMAL LOADS, CONSIDER THE MODERATE TEMPERATURE RANGE AS STIPULATED IN THE AASHTO LRFD DESIGN SPECIFICATIONS, THE NORMAL TEMPERATURE SHALL BE CONSIDERED TO BE 68F.	STRUCTURAL BACKFILL SHALL CONFORM TO THE REQUIREMENTS OF BORROW TYPE C.	548 549	1-4/
	FOR SEISMIC LOADS, CONSIDER SEISMIC PERFORMANCE ZONE 1, WITH A SITE CLASS = D AND IMPORTANCE CATEGORY - ESSENTIAL.	9. UTILITIES: COORDINATE ALL WORK RELATED TO PUBLIC AND PRIVATE UTILITIES IN ACCORDANCE WITH	550 551 552	1-47
	SEISMIC FORCES WERE CONSIDERED FOR ACCELERATION COEFFICIENT OF 0.08.	SECTION 107.04 OF THE STANDARD SPECIFICATIONS.	553	1-47
3.	PORTLAND CEMENT CONCRETE:	VERIFY AND LOCATE ALL EXISTING UTILITIES PRIOR TO STARTING WORK. CONDUCT OPERATIONS IN A MANNER WHICH ENSURES THAT THE UTILITIES WILL NOT BE DISTURBED OR	555	1-47
	PORTLAND CEMENT CONCRETE FOR CAST-IN-PLACE ELEMENTS SHALL BE AS FOLLOWS:	ENDANGERED AND ASSUME FULL RESPONSIBILITY FOR ANY DAMAGE TO UTILITIES DURING CONSTRUCTION. THE DEPARTMENT DOES NOT ASSUME RESPONSIBILITY FOR REIMBURSEMENT,	556 557	1-47
	ITEM NO. 602004 (CLASS B, f'c=3000 PSI) - ABUTMENT FOOTING ITEM NO. 602013 (CLASS D, f'c=4500 PSI) - DECK AND DIAPHRAGMS ITEM NO. 602014 (CLASS D, f'c=4500 PSI) - APPROACH SLAB, MOMENT SLAB AND SLEEPER SLAB ITEM NO. 602015 (CLASS A, f'c=4500 PSI) - ABUTMENT ABOVE FOOTING ITEM NO. 602017 (CLASS A, f'c=4500 PSI) - BARRIER RAKE FINISH ALL HORIZONTAL CONSTRUCTION, EXCEPT AS INDICATED. CONSTRUCT DECK SLAB TRANSVERSE CONSTRUCTION JOINTS PARALLEL TO BRIDGE CENTERLINE OF BEARING.	SIZE AND LOCATION OF ANY UTILITY. 10. HYDRAULIC DATA: DRAINAGE AREA = 0.46 SQ. MILES DESIGN FREQUENCY = 100 YEARS DESIGN FLOOD ELEVATION = 38.36 25-YR FLOOD ELEVATION = 37.54 DESIGN DISCHARGE (100-YR) = 973 CFS PROPOSED OPENING = 1.134 SE	559 560	1-47 1-47 1-47
	PLACE CHEEKWALL AND BACKWALL CONCRETE AFTER BEAMS HAVE BEEN SET IN POSITION.		DESIGN	
	DECK SLAB THICKNESS INCLUDES ½" INTEGRAL WEARING SURFACE.		VEHICLE	
	MIX REQUIREMENTS SHALL CONFORM TO SECTION 812 OF THE DELAWARE DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS.		HL-93 TRUCK (INVEN	TORY)
	ALL EXPOSED CORNERS OF CONCRETE SHALL BE CHAMFERED WITH 34" X 34" MILLED CHAMFER STRIPS UNLESS NOTED OTHERWISE, EXCEPT ON UNEXPOSED FOOTINGS OR WHERE INDICATED BY THE NOTATION ON THE PLANS, "DO NOT CHAMFER".		HL-93 TRUCK TRAIN (IN) HS-20 (INVENTOR)	VENTORY)
	NO SLIP-FORMING OF BARRIERS IS PERMITTED, UNLESS NOTED OTHERWISE.		HL-93 TRUCK (OPERAT	ring)
4.	BAR REINFORCEMENT:		HL-93 TANDEM (OPERA	TING)
	REINFORCING STEEL SHALL CONFORM TO AASHTO M31 (ASTM A615), GRADE 60.		HL-93 TRUCK TRAIN (OPE	ERATING
	PROVIDE 2" CONCRETE COVER ON REINFORCEMENT BARS, EXCEPT AS NOTED.		HS-20 (OPERATING	3)
	FUSION-BONDED EPOXY COATED REINFORCING STEEL SHALL CONFORM TO AASHTO M284 (ASTM D3963), AND SHALL BE DENOTED WITH A SUFFIX "E" IN THE BAR MARKS		DE S220 (LEGAL)	)
	DO NOT WELD GRADE 60 REINFORCING STEEL, UNLESS NOTED OTHERWISE.	WITH HEC-18- 'EVALUATING SCOUR AT BRIDGES' AND HEC-23- 'BRIDGE SCOUR AND STREAM	DE S335 (LEGAL)	)
5.	PRESTRESSED CONCRETE DESIGN:	WORST CASE OF THE OVERTOPPING FLOOD OR THE 500-YR FLOOD EVENT.	DE S437 (LEGAL)	)
	THE PRECAST CONCRETE GIRDERS ARE DESIGNED AS NONCOMPOSITE FOR ALL DEAD LOADS EXCEPT THE BARRIERS AND FUTURE WEARING SURFACE. THE PRECAST GIRDERS	DESIGN EVENT: 500 YR DESIGN VELOCITY: 10.7 FT/S	DE T330 (LEGAL)	)
	ARE DESIGNED AS COMPOSITE FOR LIVE LOADS AS WELL AS THE BARRIER AND FUTURE WEARING SURFACE DEAD LOADS.	DESIGN DISCHARGE: 1640 CFS DESIGN DEPTH OF FLOW: 5.3 FT	DE T435 (LEGAL)	)
			DE T540 (LEGAL)	)
			NOTE: LOAD RATING DO	DES INCL
_	ADDENDUMS /	REVISIONS	201	CONT

	ADDENDUMS	/ REVISIONS			CONT
DELAWARE					T2009
DEPARTMENT OF TRANSPORTATION			-	LEVELS RUAD	COL
·				TO SUMMIT BRIDGE ROAD	NEW (


	NDEX OF DRAWINGS
ING NO.	TITLE
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′9 AB-3	ABUTMENT 1 FOOTING REINFORCEMENT PLAN 2
'9 AB-4	ABUTMENT 1 REINFORCEMENT DETAILS 1
<b>/9 AB-5</b>	ABUTMENT 1 REINFORCEMENT DETAILS 2
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'9 DK-2	DECK AND BARRIER REINFORCEMENT
'9 DK-3	DECK DETAILS
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'9 AS-2	APPROACH SLAB DETAILS
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9 FD-2	FINISHED APPROACH SLAB ELEVATIONS
79 EX-1	EXPANSION JOINT DETAILS
79 DT-1	SETTLEMENT PLATFORM DETAIL
79 B0-1	TEST BORINGS

# LOAD RATING SUMMARY

		-			
	RATING FACTOR	RATING WEIGHT (TONS)	CONTROLLING MEMBER	CONTROLL ING POINT	LOAD EFFECT
	1.06	N/A	INTERIOR BEAM	105	CONC.STRESS MAX.EFFECTS DL+PS+LL BOT.OF GIRDER
	1.26	N/A	INTERIOR BEAM	105	CONC.STRESS MAX.EFFECTS DL+PS+LL BOT.OF GIRDER
)	N/A	N/A	N/A	N/A	N/A
	1.61	57.94	INTERIOR BEAM	105	CONC.STRESS MAX.EFFECTS DL+PS+LL BOT.OF GIRDER
	1.65	N/A	EXTERIOR BEAM	105	LONG.REINF.MAX.EFFECTS MAX. MOMENT W/ CONCURRENT SHEAR
	1.94	N/A	EXTERIOR BEAM	105	LONG.REINF.MAX.EFFECTS MAX. MOMENT W/ CONCURRENT SHEAR
)	N/A	N/A	N/A	N/A	N/A
	2.18	78.37	INTERIOR BEAM	106	LONG.REINF.MIN.EFFECTS MAX. SHEAR W/ CONCURRENT MOMENT
	2.25	44.93	INTERIOR BEAM	105	CONC.STRESS MAX.EFFECTS DL+PS+LL BOT.OF GIRDER
	1.26	44.25	INTERIOR BEAM	105	CONC.STRESS MAX.EFFECTS DL+PS+LL BOT.OF GIRDER
	1.20	44.13	INTERIOR BEAM	105	CONC.STRESS MAX.EFFECTS DL+PS+LL BOT.OF GIRDER
	1.63	49.01	INTERIOR BEAM	105	CONC.STRESS MAX.EFFECTS DL+PS+LL BOT.OF GIRDER
	1.42	49.64	INTERIOR BEAM	105	CONC. STRESS MAX. EFFECTS DL+PS+LL BOT. OF GIRDER
	1.25	49.94	INTERIOR BEAM	105	CONC. STRESS MAX. EFFECTS DL+PS+LL BOT. OF GIRDER
.[	JDE FUTUR	E WEARI	NG SURFACE AS	NOTED IN THE	PLANS.

				1-479 GN-1
RACT	BRIDGE NO.	1_479		SHEET NO.
911303			SANDY BRANCH	524
NTY	DESIGNED BY:	££	GENERAL NOTES AND	TOTAL SHTS.
CASTLE	CHECKED BY:	JW	INDEX OF DRAWINGS	1256

EST	IMATED BRIDGE QUANT	- I T I E	ES
ITEM NUMBER	DESCRIPTION	UNIT	OUANTI
202505	SETTLEMENT PLATFORM	EACH	2
202518	SETTLEMENT MONUMENT	ЕАСН	2
207000	EXCAVATION AND BACKFILL FOR STRUCTURES	CY	559
302012	DELAWARE NO. 57 STONE	TON	13
602004	PORTLAND CEMENT CONCRETE MASONRY, ABUTMENT FOOTING, CLASS B	CY	252
602013	PORTLAND CEMENT CONCRETE MASONRY, SUPERSTRUCTURE, CLASS D	CY	225
602014	PORTLAND CEMENT CONCRETE MASONRY, APPROACH SLAB, CLASS D	CY	209
602015	PORTLAND CEMENT CONCRETE MASONRY, ABUTMENT ABOVE FOOTING, CLASS A	CY	341
602017	PORTLAND CEMENT CONCRETE MASONRY, PARAPET, CLASS A	CY	58
603000	BAR REINFORCEMENT	LB	36,8 <mark>8</mark>
604000	BAR REINFORCEMENT, EPOXY COATED	LB	126, <mark>8</mark> 6
605512	PREFABRICATED EXPANSION JOINT SYSTEM, 4"	LF	95
605581	ELASTOMERIC BRIDGE BEARING PAD	EA	10
618041 (AL TERNATE)	FURNISH CAST-IN-PLACE CONCRETE PILES, 14"	LF	5,25
618046 (ALTERNATE)	FURNISH CAST-IN-PLACE CONCRETE TEST PILES, 14"	LF	249
618081	FURNISH PRECAST PRESTRESSED CONCRETE PILE, 14"×14"	LF	6,07
618091	FURNISH PRECAST PRESTRESSED CONCRETE TEST PILE, 14"×14"	LF	281
619021 (ALTERNATE)	INSTALL CAST-IN-PLACE CONCRETE PILES, 14"	LF	5,25
619025 (ALTERNATE)	INSTALL CAST-IN-PLACE CONCRETE TEST PILES, 14"	LF	249
619061	INSTALL PRECAST PRESTRESSED CONCRETE PILE, 14"×14"	LF	6,07
619067	INSTALL PRECAST PRESTRESSED CONCRETE TEST PILE, 14"x14"	LF	281
619501	PRODUCTION PILE RESTRIKE	ЕАСН	6
619502	TEST PILE RESTRIKE	EADY	4
619519	DYNAMIC PILE TESTING BY CONTRACTOR	ЕАСН	4
619539	SIGNAL MATCHING ANALYSIS BY CONTRACTOR	ЕАСН	4
623003	PRESTRESSED REINFORCED CONCRETE MEMBERS, BULB TBEAM	LS	-
712006	RIPRAP, R-5	SY	141
71 300 3	GEOTEXTILE, RIPRAP	SY	141
733001	TOPSOILING, 4" DEPTH	SY	282







	110 004	CON
0 2 8	US 301	T2009
1 4	LEVELS ROAD	COU
 FEET	TO SUMMIT BRIDGE ROAD	NEW (

		RAMP F	SUPERELEVATION					
--	--	--------	----------------	--	--	--	--	--

# 2' -0" 0FFSET 1′ -5<u>¼″</u> BARRIER SHLD. ∽ 3″ RGS CONDUIT 4% PCEF 7147 PRESTRESSED CONCRETE GIRDER (TYP.) 65 3' -5<sup>1</sup>/4" CROSS REFERENCE NOTES: 1. FOR GIRDER PLAN AND ELEVATION, SEE DWG. NO. 1-479 BM-1.

- 2. FOR GIRDER DETAILS, SEE DWG. NO. 1-479 BM-2.
- 3. FOR INTERMEDIATE AND END DIAPHRAGM DETAILS, SEE DWG. NO. 1-479 DPH-1.
- FOR DECK AND BARRIER REINFORCEMENT, SEE DWG. NO. 1-479 DK-2.
- 5. FOR DECK DETAILS, SEE DWG. NO. 1-479 DK-3.
- 6. FOR FINISHED DECK ELEVATIONS, SEE DWG. 1-479 FD-1.
- 7. FOR GENERAL PLAN AND ELEVATION, SEE DWG. NO. 1-479 PE-1.
- 8. FOR CONDUIT DETAILS, SEE DWG. NO. DT-07.

				1-479 TS-1	
RACT	BRIDGE NO.	1_479		SHEET NO.	
911303			SANDY BRANCH		
INTY	DESIGNED BY: LT		TYPICAL SECTION	TOTAL SHTS.	
CASTLE	CHECKED BY: JW		AND QUANTITIES	1256	



				1	1-4/9 11-1
SCALE	110 004	CONTRACT	BRIDGE NO. <b>1_479</b>	BAMP E OVER	SHEET NO.
	US 301	T200911303			526
	LEVELS BOAD		DESIGNED BY: EE		
1 4		COUNTY			TOTAL SHTS.
FEET	TO SUMMIT BRIDGE ROAD	NEW CASTLE	CHECKED BY: JW	GEOIVIETRIC LATOUT	1256

![](_page_3_Figure_4.jpeg)

![](_page_4_Figure_0.jpeg)

						1-479 FT-2
	110 004	CONTRACT	BRIDGE NO.	1_479		SHEET NO.
08	US 301	T200911303		1 470	SANDY BRANCH	527
1 4	LEVELS ROAD	COUNTY	DESIGNED BY:	LT		TOTAL SHTS.
FEET	TO SUMMIT BRIDGE ROAD	NEW CASTLE	CHECKED BY:	JW	ABUTMENT 1 PILE PLAN	1256

PILE	INSTALLA	TION DATA	
DESIGN DA	ATA	ACTUAL FI	ELD DATA
DRIVING (KIP)	ESTIMATED TIP ELEVATION	AVERAGE MINIMUM TIP ELEVATION	AVERAGE MAXIMUM TIP ELEVATION
	-20.00		
ΔΤΙΟ	Ν ΠΔΤΔ (Ν	IONOTURE ALT	FRNATE)

		NONOTODE ALT		
DESIGN DATA		ACTUAL FIELD DATA		
DRIVING (KIP)	ESTIMATED TIP ELEVATION	AVERAGE MINIMUM TIP ELEVATION	AVERAGE MAXIMUM TIP ELEVATION	
	-18.00			

LE DRIVING INFORMATION
AND TYPE:
ARING OBTAINED:
ACTUAL BLOWS/FT.:
MER ENERGY:
PILE LEGEND
BATTERED PILE
[] PLUMB PILE
AND TEST PILE
CROSS REFERENCE NOTES:
1. FOR GENERAL PLAN AND ELEVATION, SEE DWG. NO. 1-479 PE-1.

- 2. FOR ABUTMENT 1 PLAN AND ELEVATION, SEE DWG. NO. 1-479 AB-1.
- 3. FOR ABUTMENT 1 TYPICAL SECTION, SEE DWG. NO. 1-479 AB-4.
- 4. FOR PILE DETAILS, SEE DWG. NO. 1-479 PL-1.
- 5. FOR FOOTING REINFORCEMENT, SEE DWG. NOS. 1-479 AB-2 AND 1-479 AB-3.
- 6. FOR WORKING POINT COORDINATES, SEE DWG. NO. 1-479 FT-1.

![](_page_5_Figure_0.jpeg)

D	ELA	WARE
DEPARTMENT	OF	TRANSPORTATIO

![](_page_5_Figure_3.jpeg)

PILE	INSTALLA	TION DATA	
DESIGN D	ΔΤΑ	ACTUAL FI	ELD DATA
AL PILE DRIVING ISTANCE (KIP)	ESTIMATED TIP ELEVATION	AVERAGE MINIMUM TIP ELEVATION	AVERAGE MAXIMUM TIP ELEVATION
377	- 30. 00		
<b>ALLATION</b>	N DATA (N	IONOTUBE ALTE	ERNATE)
	<b>ΔΤΔ</b>		

DESIGN D	ATA	ACTUAL FIE	ELD DATA
AL PILE DRIVING ISTANCE (KIP)	ESTIMATED TIP ELEVATION	AVERAGE MINIMUM TIP ELEVATION	AVERAGE MAXIMUM TIP ELEVATION
377	-16.00		

				1-479 FT-3
TRACT	BRIDGE NO.	1-479	RAMP E OVER	SHEET NO.
911303		1 470	SANDY BRANCH	528
UNTY	DESIGNED BY:			TOTAL SHTS.
CASTLE	CHECKED BY:	JW	ABUTMENT 2 PILE PLAN	1256

![](_page_6_Figure_0.jpeg)

PILE NOTES:

- 1. ALL PILES SHALL BE 14"X14" PRESTRESSED PRECAST CONCRETE PILES. PILES SHALL NOT BE COATED.
- 2. THE MINIMUM COMPRESSIVE STRENGTH FOR THE PRESTRESSED CONCRETE PILES AT THE AGE OF 28 DAYS SHALL BE f'c=6000 PSI. THE MINIMUM COMPRESSIVE STRENGTH AT THE TIME OF TRANSFER OF PRESTRESS SHALL BE f'c1=4800 PSI.
- 3. PRETENSIONING STEEL FOR PILES SHALL CONSIST OF 1/2" DIAMETER 7-WIRE LOW RELAXTION STRANDS CONFORMING TO THE REQUIREMENTS OF AASHTO M203 (ASTM A416) GRADE 270. EACH 1/2" DIAMETER STRAND SHALL BE PRETENSIONED TO 30,982 LBS.(0.75 Fpu).
- 4. SPIRAL TIES SHALL BE #5 GAGE STEEL WIRE CONFORMING TO THE REQUIREMENTS OF SECTION 618.18.
- 5. THE SPLICING OF PRESTRESSED PRECAST CONCRETE PILES SHALL NOT BE PERMITTED.
- 6. A MINIMUM QUARANTINE PERIOD OF 30 DAYS IS REQUIRED AFTER THE CONSTRUCTION OF THE FULL HEIGHT OF THE FILL AT THE ABUTMENTS IS ACHIEVED.
- 7. PILES MAY NOT BE DRIVEN UNTIL AFTER THE COMPLETION OF THE 30 DAY OUARANTINE PERIOD.
- 8. ALL TEST PILES SHALL BE 10 FT LONGER THAN PRODUCTION PILES AS INDICATED IN THE PILE INSTALLATION DATA TABLE.
- 9. THE ENGINEER SHALL APPROVE THE COMPLETION OF THE WAITING PERIOD, BASED ON RESULTS OF INSTRUMENTATION.
- 10. TEST PILES MAY BE DRIVEN PRIOR TO PLACING EMBANKMENT AND SURCHARGE MATERIAL. RESTRIKES OF THESE TEST PILES SHALL BE PERFORMED PRIOR TO PLACING EMBANKMENT IN ACCORDANCE WITH ITEM NO. 619502 - TEST PILE RESTRIKE. AFTER THE SETTLEMENT HAS BEEN ACHIEVED AND THE SUBSTRUCTURE HAS BEEN RELEASED BY THE ENGINEER, PRODUCTION PILES MAY BE INSTALLED. AT THIS POINT, THE TEST PILE SHALL BE ACTING AS A PRODUCTION PILE AND IT SHALL BE RE-STRUCK PRIOR TO PLACING ANY OTHER PRODUCTION PILES WITH PAYMENT UNDER ITEM NO. 619501 - PRODUCTION PILE RESTRIKE.
- 11. SEE SPECIAL PROVISIONS 202505 AND 202518 FOR SETTLEMENT MONITORING LOCATIONS AND REQUIREMENTS.
- 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING A WAVE EQUATION ANALYSIS AND ALL OTHER INCIDENTALS IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. THE WAVE EQUATION AND HIGH-STRAIN DYNAMIC PILE TESTING MUST BE SIGNED AND STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF DELAWARE IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
- 13. UPON COMPLETION OF THE HIGH-STRAIN DYNAMIC PILE TESTING THE CONTRACTOR SHALL SUBMIT A SIGNAL MATCHING ANALYSIS TO THE ENGINEER FOR REVIEW AND APPROVAL IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
- 14. ALL PILES SHALL BE DRIVEN TO THE NOMINAL PILE DRIVING RESISTANCE LISTED IN THE PILE INSTALLATION DATA TABLE.
- 15. DELDOT STANDARD SPECIFICATION 619.11(0)(6) SHALL BE MODIFIED BY REFERENCE TO SPECIAL PROVISIONS 619519 AND 619539.

16. PILE LENGTHS FOR ORDERING PURPOSES SHALL BE DETERMINED BY THE TEST PILES. A MINIMUM OF ONE PILE PER SUBSTRUCTURE, AS SHOWN ON THE PLANS, SHALL BE DYNAMICALLY TESTED WITH SIGNAL MATCHING ANALYSIS BY THE CONTRACTOR IN ACCORDANCE WITH SPECIAL PROVISIONS 619519 AND 619539. TEST AND PRODUCTION PILE RESTRIKES WILL BE PAID AS FOLLOWS:
A. ALL TEST PILE(S) WILL BE RESTRUCK AFTER A WAITING PERIOD OF AT LEAST 48 HOURS. TEST PILE RESTRIKES SHALL BE INCIDENTAL TO THE INITIAL INSTALLATION OF THE PILE PROVIDED THEY ARE REQUESTED WITHIN FIVE WORKING DAYS FROM THE COMPLETION OF THE INITIAL DRIVE. IF TEST PILE RESTRIKES ARE REQUESTED AFTER THE FIVE WORKING DAYS FROM THE COMPLETION OF THE INITIAL DRIVE, THE TEST PILE RESTRIKE SHALL BE PAID AS NOTED IN SPECIAL PROVISION 619502.
B. IF DIRECTED BY THE ENGINEER TO RESTRIKE A PRODUCTION PILE, THE RESTRIKE OF THE PRODUCTION PILE SHALL BE PAID SEPARATELY UNDER ITEM NO. 619501.

17. THE DEPARTMENT RESERVES THE RIGHT TO PERFORM DYNAMIC TESTING OF RESTRIKES.

EXISTING GROUNDL INE

DETAIL SIMILAR FOR WINGWALLS.

- 1. FOR ABUTMENT 1 PILE PLAN, SEE DWG. NO. 1-479 FT-2.
- 2. FOR ABUTMENT 2 PILE PLAN, SEE DWG. NO. 1-479 FT-3.

				1-479 PL-1
RACT	BRIDGE NO.	1_479		SHEET NO.
11303		1 470	SANDY BRANCH	529
NTY	DESIGNED BY:	LT		TOTAL SHTS.
ASTLE	CHECKED BY:	JW	FILE DETAILS	1256

![](_page_7_Figure_0.jpeg)

RACT	BRIDGE NO	1_/170		SHEET NO.
911303		1-4/3	SANDY BRANCH	530
INTY	DESIGNED BY:		ABUTMENT 1 PLAN	TOTAL SHTS.
CASTLE	CHECKED BY:	JW	AND ELEVATION	1256

![](_page_8_Figure_0.jpeg)

		ADDENDUMS	/ REVISIONS
	DELAWARE		
DEPARTMENT OF TRANSPORTATION			

					1-479 AB-2
	110 004	CONTRACT	BRIDGE NO. <b>1_479</b>		SHEET NO.
	US 301	T200911303			531
	LEVELS ROAD	COUNTY	DESIGNED BY: LT		TOTAL SHTS.
FEET	TO SUMMIT BRIDGE ROAD	NEW CASTLE	CHECKED BY: JW	REINFORCEMENT PLAN 1	1256

![](_page_8_Picture_3.jpeg)

![](_page_8_Picture_4.jpeg)

- 1. FOR ABUTMENT 1 PILE PLAN, SEE DWG. NO. 1-479 FT-2.
- 2. FOR ABUTMENT 1 PLAN AND ELEVATION, SEE DWG. NO. 1-479 AB-1.
- 3. FOR REINFORCING BAR SCHEDULE, SEE DWG. NO. 1-479 BR-1.
- 4. FOR WORKING POINT COORDINATES, SEE DWG. NO. 1-479 FT-1.

![](_page_9_Figure_0.jpeg)

					1-479 AB-3
	110 001	CONTRACT	BRIDGE NO. <b>1_479</b>		SHEET NO.
0 2 8		T200911303		SANDY BRANCH	532
1 4	LEVELS KOAD	COUNTY	DESIGNED BY: LI	ABUTMENT 1 FOOTING	TOTAL SHTS.
FEET	TO SUMMIT BRIDGE ROAD	NEW CASTLE	CHECKED BY: JW	<b>REINFORCEMENT PLAN 2</b>	1256
					<u></u>

![](_page_9_Picture_2.jpeg)

![](_page_9_Picture_3.jpeg)

- 1. FOR ABUTMENT 1 PILE PLAN, SEE DWG. NO. 1-479 FT-2.
- 2. FOR ABUTMENT 1 PLAN AND ELEVATION, SEE DWG. NO. 1-479 AB-1.
- 3. FOR REINFORCING BAR SCHEDULE, SEE DWG. NO. 1-479 BR-1.
- 4. FOR WORKING POINT COORDINATES, SEE DWG. NO. 1-479 FT-1.

![](_page_10_Figure_0.jpeg)

110 001	CONT
	T2009
LEVELS ROAD	COL
TO SUMMIT BRIDGE ROAD	NEW (

![](_page_10_Figure_5.jpeg)

![](_page_11_Figure_0.jpeg)

				1-479 AB-5
TRACT	BRIDGE NO.	1_479		SHEET NO.
044707	5	1 4/5		<b>F7</b> 4
911303	DESIGNED DV.	ΙT	SANDY BRANCH	534
JNTY	DESIGNED DI-	LI	ABUTMENT 1	TOTAL SHTS.
CASTLE	CHECKED BY:	JW	REINFORCEMENT DETAILS 2	1256

![](_page_12_Figure_0.jpeg)

# NOTES:

- 1. TOP OF ABUTMENT SEAT ELEVATIONS SHOWN ARE AT THE FRONT FACE OF BACKWALL.
- 2. TOP OF BACKWALL ELEVATIONS SHOWN AT CENTER OF BACKWALL.

- 1. FOR GENERAL PLAN AND ELEVATION, SEE DWG. NO. 1-479 PE-1.
- 2. FOR GEOMETRIC LAYOUT, SEE DWG. NO. 1-479 FT-1.
- 3. FOR PILE LAYOUT AT ABUTMENT 2, SEE DWG. NO. 1-479 FT-3.
- 4. FOR SECTION F-F, SEE DWG. NO. 1-479 AB-9.
- 5. FOR ABUTMENT DETAILS, SEE DWG. NOS. 1-479 AB-9 AND 1-479 AB-10.
- 6. FOR WINGWALL ELEVATIONS, SEE DWG. NO. 1-479 WW-1.
- 7. FOR BEARING DETAILS, SEE DWG. NO. 1-479 BD-1.
- 8. FOR CONSTRUCTION JOINT DETAIL, SEE DWG. NO. 1-479 AB-1.
- 9. FOR STEM EXPANSION JOINT DETAIL, SEE DWG. NO. 1-479 AB-10.

				1-479 AB-6
RACT	BRIDGE NO.	1_479		SHEET NO.
11303		1475		5 3 5
11303	DESIGNED DV.	L T	SANDY BRANCH	555
INTY	DESIGNED DI-		ABUTMENT 2 PLAN	TOTAL SHTS.
CASTLE	CHECKED BY:	JW	AND ELEVATION	1256

![](_page_13_Picture_0.jpeg)

		ADDENDUMS	/ REVISIONS
DELAWARE DEDADTMENT OF TRANSPORTATION			

![](_page_13_Figure_4.jpeg)

![](_page_13_Picture_5.jpeg)

![](_page_13_Picture_6.jpeg)

- 1. FOR ABUTMENT 2 PILE PLAN, SEE DWG. NO. 1-479 FT-3.
- 2. FOR ABUTMENT 2 PLAN AND ELEVATION, SEE DWG. NO. 1-479 AB-6.
- 3. FOR REINFORCING BAR SCHEDULE, SEE DWG. NO. 1-479 BR-2.
- 4. FOR WORKING POINT COORDINATES, SEE DWG. NO. 1-479 FT-1.

				1-479 AB-7
FRACT	BRIDGE NO.	1_479		SHEET NO.
911303		1-4/3	SANDY BRANCH	536
JNTY	DESIGNED BY:		ABUTMENT 2 FOOTING	TOTAL SHTS.
CASTLE	CHECKED BY:	JW	<b>REINFORCEMENT PLAN 1</b>	1256

![](_page_14_Figure_0.jpeg)

						1-479 AB-8
		CONTRACT	BRIDGE NO.	1_479		SHEET NO.
0 2 8	US 301	T200911303		1-475	SANDY BRANCH	537
- 1 4	LEVELS ROAD	COUNTY	DESIGNED BY:	LT		TOTAL SHTS.
- FEET	TO SUMMIT BRIDGE ROAD	NEW CASTLE	CHECKED BY:	JW	REINFORCEMENT PLAN 2	1256
FEET	LEVELS ROAD TO SUMMIT BRIDGE ROAD	COUNTY NEW CASTLE	DESIGNED BY: CHECKED BY:	LT JW	- ABUTMENT 2 FOOTING REINFORCEMENT PLAN 2	TOTAL SHTS 1256

![](_page_14_Picture_3.jpeg)

![](_page_14_Picture_4.jpeg)

- 1. FOR ABUTMENT 2 PILE PLAN, SEE DWG. NO. 1-479 FT-3.
- 2. FOR ABUTMENT 2 PLAN AND ELEVATION, SEE DWG. NO. 1-479 AB-6.
- 3. FOR REINFORCING BAR SCHEDULE, SEE DWG. NO. 1-479 BR-2.
- 4. FOR WORKING POINT COORDINATES, SEE DWG. NO. 1-479 FT-1.

![](_page_15_Figure_0.jpeg)

	ADDENDUMS	/ REVISIONS
DELAWARE		
DEPARTIVIENT OF TRANSPORTATION		

110,004	CONT
	T2009
	COL
I TO SUMMIT BRIDGE ROAD	NEW (

				1 173 76 3
RACT	BRIDGE NO.	1_479		SHEET NO.
11303		1-475	SANDY BRANCH	538
NTY	DESIGNED BY:	LT	ABLITMENT 2	TOTAL SHTS.
ASTLE	CHECKED BY:	JW	<b>REINFORCEMENT DETAILS 1</b>	1256

![](_page_16_Figure_0.jpeg)

		CONT
		T2009
	LEVELS KUAD	COUN
	TO SOMMUT BRIDGE ROAD	NEW C

- 2. REINFORCEMENT SHALL NOT PASS THROUGH EXPANSION JOINT.
- 3. ONLY PLACE EXPANSION JOINT IN STEM (NO JOINT IN FOOTING).

- 1. FOR ABUTMENT 2 PLAN AND ELEVATION, SEE DWG. NO. 1-479 AB-6.

				1-479 AB-10
RACT	BRIDGE NO.	1_479		SHEET NO.
11303		1 470	SANDY BRANCH	539
NTY	DESIGNED BY:		ABUTMENT 2	TOTAL SHTS.
ASTLE	CHECKED BY:	JW	<b>REINFORCEMENT DETAILS 2</b>	1256

![](_page_17_Figure_0.jpeg)

![](_page_18_Figure_0.jpeg)

	-
	CONTE
US 301	T2009
LEVELS ROAD	COUN
TO SUMMIT BRIDGE ROAD	NEW C

![](_page_18_Figure_4.jpeg)

![](_page_19_Figure_0.jpeg)

	US 301	Т20
	LEVELƏ NVAV	(
	TO CLIMMIT DDIDCE DOAD	
	IU SUIVIIVIII DRIDGE RUAD	NEW

ABUINCN 1         ABUINCN 1         ABUINCN 1         ABUINCN 1         C         Note that the second colspan="2">C         Note the second colspan="2">ABUINCN 1           THE INFORMATION IN THE INFORMATION INFORMA				BAR SCHEDULE	REINFORCING									
Inter     And     No.     No.     P.		ABUTMENT 1						ABUTMENT 1						
Set:	H J K O REMARKS	D E F G H	A B C D	MARK LENGTH NO. BARS TYPE	REMARKS	КО	G H	E F G	C D	В	(PE A	NO. BARS TY	LENGTH N	MARK
Solid Set       C       2       9       100*       2-2       9       100*       2-2       100       2-2       100       100* <td></td> <td></td> <td>1'-2" 4'-11"</td> <td>F701 6'-1" 63 2</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>20' -9"</td> <td>TR. 2 10"</td> <td>6 S1</td> <td>9' -0" 21' -7"</td> <td>A501E A502E</td>			1'-2" 4'-11"	F701 6'-1" 63 2						20' -9"	TR. 2 10"	6 S1	9' -0" 21' -7"	A501E A502E
302       302       302       1 <th1< th="">       1       1       <th1< t<="" td=""><td><math display="block">\triangle = 2' - 10''</math></td><td></td><td></td><td>F702 <math>9'-2''</math> 3 STR.</td><td></td><td></td><td></td><td></td><td></td><td>18' -7"</td><td>2 10"</td><td>6</td><td>19' -5"</td><td>A503E</td></th1<></th1<>	$\triangle = 2' - 10''$			F702 $9'-2''$ 3 STR.						18' -7"	2 10"	6	19' -5"	A503E
nome     no     no <td></td> <td></td> <td></td> <td>F703 12'-0" 62 STR.</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>TR. TR.</td> <td>11 ST 2 ST</td> <td>46' -8"</td> <td>A504</td>				F703 12'-0" 62 STR.							TR. TR.	11 ST 2 ST	46' -8"	A504
830     830 <td><math display="block">\triangle = 2' - 8\frac{1}{8''}</math></td> <td></td> <td></td> <td><math>F704 = 11' - 5\frac{1}{2}'' = 5</math> STR.</td> <td></td> <td><u> </u></td> <td></td> <td></td> <td>1'-9" 1'-9"</td> <td></td> <td>T7</td> <td>12 T</td> <td>3' -6"</td> <td>A506</td>	$\triangle = 2' - 8\frac{1}{8''}$			$F704 = 11' - 5\frac{1}{2}'' = 5$ STR.		<u> </u>			1'-9" 1'-9"		T7	12 T	3' -6"	A506
Sold	71⁄4″	6'-1"	1'-2" 6'-1"	F 705 7' - 3" 55 T7	2"	<u> </u>	1' -8"		1'-9" 1'-9"		16 TR.	11 1 12 ST	3' -6" 40' -9"	A507 A508
BASE		3' -6"	2'-1" 1'-2" 3'-6"	W501 6'-9" 55 17							TR.	63 S1	8' -0"	A509
NOVE       Prime       No				<b>W502</b> $26' - 6\frac{1}{2}''$ 31 STR.						4' - 4"	2 10"	63	5' - 2"	A510E
08.98       6.95       7.97       100       2.91       1.92       2.91       1.92				$w_{503}$ $23^{-3}/2$ $31^{-1}$ $31^{-1}$ $w_{504}$ $14' - 4\frac{1}{2}''$ 28         STR.							TR.	102 S1	5' -10 <sup>1</sup> / <sub>2</sub> "	A512E
sing       drig				₩505 13'-4½" 27 STR.					1/_2/ 2/_1/	<u> </u>	TR.	46 ST	4' -0"	A513E
64-56       60-57       7				W701 14'-11" 28 STR.						2 -1	TR.	7 S1	44' -9"	A514E
Nome     Nome     I     0     I <t< td=""><td></td><td></td><td></td><td>W702 13'-11" 27 STR.</td><td>/ "</td><td></td><td>75/#</td><td></td><td></td><td></td><td>TR.</td><td>7 SI</td><td>40' -9"</td><td>A516E</td></t<>				W702 13'-11" 27 STR.	/ "		75/#				TR.	7 SI	40' -9"	A516E
Marge       r-g       1 <th1< th="">       1       <th1< th=""> <th1< td="" th<=""><td></td><td></td><td></td><td></td><td>2</td><td><u> </u></td><td><u>ວ%</u></td><td></td><td><u>2'-1" 1'-2"</u> 2'-8" 1'-7"</td><td>1′-7″</td><td>17</td><td>/ 1 4 1</td><td><u> </u></td><td>A517E</td></th1<></th1<></th1<>					2	<u> </u>	<u>ວ%</u>		<u>2'-1" 1'-2"</u> 2'-8" 1'-7"	1′-7″	17	/ 1 4 1	<u> </u>	A517E
A1201       A1201 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>3' - 4" 1' - 7"</td><td>1'-7"</td><td>17</td><td>4 1</td><td>6' -6"</td><td>A519E</td></td<>									3' - 4" 1' - 7"	1'-7"	17	4 1	6' -6"	A519E
622       14'-9'       4       176,       1 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>IR.</td><td>13 ST 12 ST</td><td>7' - 2" 7' - 3<sup>1</sup>/<sub>2</sub>"</td><td>A520 A521</td></t<>											IR.	13 ST 12 ST	7' - 2" 7' - 3 <sup>1</sup> / <sub>2</sub> "	A520 A521
bcb       brow       4       30b       1											TR.	4 S1	14' -9"	A522
Avgce       19-10 <sup>2</sup> 4       5R,											IR.	4 S1 4 S1	15 <sup>.</sup> -10" 14' -9"	A523 A524E
Above       Y - 2 <sup>+</sup> 8       3He       I											TR.	4 S1	13' -10"	A525E
Above     Py-P     7     STR.     Py-P     PsK/P											TR. TR.	8 S1 14 S1	<u>7'-3"</u> 4'-6"	A526E A527E
Above       7 - 3/2'       7       7 is 1       1' - 10'       2' - 1'       1' - 3/2'       5'', 4' - 0'/2'       1											TR.	7 S1	7' -2"	A528E
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $										)" 2' - 1 "	TR. 2 1'-10"	7 SI	$7' - 3\frac{1}{2}''$ 3' - 1 1 ''	A529E
A532E       5'-8''       14       16       12       2'-1''       1'-9''       2'-1''       2'-0'/c       6'/2       4'-1'/2''         A532E       4'-9'       55       51R       -<					8	5 <sup>3</sup> / <sub>8</sub> " 4' -0 <sup>3</sup> / <sub>8</sub> "	1'-8¼"		3' - 7" 1' - 9"		16	12 1	5' -8"	A531
ALGG       3-5       1       6       1 </td <td></td> <td></td> <td></td> <td></td> <td>/2"</td> <td><u> </u></td> <td>2' -0¼"</td> <td></td> <td>3' - 7" 2' - 1"</td> <td>)" 1'-0"</td> <td>16 2 11 - 10"</td> <td>14 1</td> <td>5' - 8"</td> <td>A532E</td>					/2"	<u> </u>	2' -0¼"		3' - 7" 2' - 1"	)" 1'-0"	16 2 11 - 10"	14 1	5' - 8"	A532E
A700 $8 \cdot 6^{\circ}$ $56$ $STR$ STR       STR <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td><u> </u></td> <td>Z 1 - 10 TR.</td> <td>56 S1</td> <td>4' -9"</td> <td>A5555 A542E</td>										<u> </u>	Z 1 - 10 TR.	56 S1	4' -9"	A5555 A542E
A/01 $6^{+}$ · · · · · · · · · · · · · · · · · · ·												F.C. (1)	01 01	1701
A03       14'-4'       4       SIR.       SIR.       SIR.         501       4'-5'       134       SIR.       SIR.       SIR.       SIR.         503       4'-7'       21       SIR.       SIR.       SIR.       SIR.         503       4'-7'       21       SIR.       SIR.       SIR.       SIR.       SIR.         504       3'-7'       2       SIR.       SIR.       SIR.       SIR.       SIR.       SIR.         505       3'-7'       2       SIR.							_				TR.	<u> </u>	<u>8' - 6"</u> 15' - 3"	A701 A702
F50       4' -5'       134       STR.       Image: Constraint of the straint of the stra											TR.	4 S1	14' -4"	A703
5902       49'-4"       4       SIR       Image: Sir and simple sim											TR.	134 S1	4' -5"	F501
$r_{303}$ $sy_{3}$ $r_{21}$ $sin_{11}$											TR.	4 S1	49' - 4"	F502
F505     34'-1'     2     STR.											TR.	6 S1	<u>49</u> - 7 36' - 7"	F503
F506       35° 0''       2       SIR       Image: state sta											TR.	2 S1	34' -1"	F505
$F507$ $27' \cdot 8''$ $8$ $STR.$ $I$ </td <td></td> <td></td> <td></td> <td></td> <td><math display="block">\triangle = 1' - 6''</math></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>TR.</td> <td>2   51</td> <td>ວວ⁻-Ⴆ″ 10 35′-0″</td> <td>F506</td>					$\triangle = 1' - 6''$						TR.	2   51	ວວ⁻-Ⴆ″ 10 35′-0″	F506
r-ue       2       510       2       511       25       511       25       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1       511       25       1 <th1< th="">       1       1       1</th1<>											TR.	8 S1	27' -8"	F507
$r 309$ $34' \cdot 4''$ $2$ $51R$ $a = 1$ $a = 1^{12} \cdot 6''$ F511 $25' - 1''$ $6$ $STR$ $a = 1$ $a = 1^{12} \cdot 6''$ F512 $25' - 1''$ $6$ $STR$ $a = 1$ $a = 1^{12} \cdot 6''$ F513 $23' - 3''$ $11$ $STR$ $a = 1$ $a = 1^{12} \cdot 6''$ F514 $34' - 4''$ $1$ $STR$ $a = 1$					LENGTH						IR. TD		<u>22 -0</u> 32' -10" TO	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $					△ = 1′-6″						IR. TD	2 5	34' - 4"	
F512       35'-0"       1       STR.       Image: constraint of the stress of the strest											TR.	4 ST 6 ST	<u>32' - 10"</u> 25' - 1"	F511
Horizontal Sine     Sine     Sine     Sine     Sine       F514     34' - 4"     1     Sine     Sine     Sine       F515     25' - 8"     11     Sine     Sine     Sine       F601     10' - 3"     10' - 3"     Sine     Sine       F602     12' - 0"     35     Sine     Sine       F603     9' - 6"     5     Sine     Sine       F604     6' - 1"     5     Sine     Sine											TR.	1 S1	35' -0"	F512
F515 $25' - 8''$ 11       STR.         F601 $10' - 3''$ 10 $6$ STR.         F601 $10' - 3''$ 10 $6$ STR.         F602 $12' - 0''$ 35       STR.       Image: Stress of the stres of the stress of the stress of the stres											IR. TR.	11 ST 1 ST	23' - 3" 34' - 4"	F513 F514
F601 $10' - 3'' \cdot 10' \\ 12' - 0'' \\ 12' - 0'' \\ 12' - 0'' \\ 5 \\ 5 \\ 5 \\ 5 \\ 5 \\ 5 \\ 5 \\ 5 \\ 5 \\$											TR.	11 S1	25' -8"	F515
F601 $12' \cdot 0''$ 6SIR.6SIR.F602 $12' \cdot 0''$ 35SIR.Image: Single state stat													10' - 2" TO	
F602       12'-0"       35       STR.       Image: STR.					$\triangle = 4\%''$						TR.	6 S1	12' -0"	F601
F604     6' -1"     5     STR.     6     7     7     7     8     7     7     8     7     7     8     7     7     7     8     7      7     7											TR.	35 S1	12' -0"	F602
											TR.	55	<u> </u>	F604
F605 $\frac{81/2"}{3'-31/4"}$ 2 STR.											TR.	2 S1	8 <sup>1</sup> / <sub>2</sub> " TO	F605
$\frac{3 - 3}{2}$ F606 4' - 3" 5 STR.					$\Delta - Z - I^{*}$						TR.	5 S1	4' - 3"	F606
F607       10' -6"       128       STR.       Image: STR in the state of the stat											TR.	128 S1	10' -6"	F607
F608 6'-11" 10 STR.											IR.	10   S1	6' -11"	F608

![](_page_20_Picture_1.jpeg)

ADDENDUMS / REVISIONS

		CONT
	US 301	T2009
	LEVELS ROAD	COL
	TO SUMMIT BRIDGE ROAD	NEW (

![](_page_20_Figure_4.jpeg)

![](_page_20_Figure_5.jpeg)

![](_page_20_Figure_6.jpeg)

![](_page_20_Figure_7.jpeg)

				1-479 BR-1
TRACT	BRIDGE NO.	1_479		SHEET NO.
011303	811802 1101	1475		513
911303	DESIGNED DV.	ΙT	SANDY BRANCH	545
JNTY	DESIGNED DI-		ABUTMENT 1	TOTAL SHTS.
CASTLE	CHECKED BY:	BCM	REINFORCEMENT SCHEDULE	1256

REINFORCING	BAR SCHEDULE
ABUTMENT 2	ABUTMENT 2
MARK LENGTH NO. BARS TYPE A B C D E F G H J K O REMARKS	MARK LENGTH NO. BARS TYPE A B C D E F G H J K O REMARKS
A504       46' - 8"       12       STR.         A506       3' - 6"       12       T7       1' - 9"       1' - 9"	F701         6' - 1"         63         2         1' - 2"         4' - 11"         1 <th1< th=""> <th1< th=""> <th1< th=""> <th1< th=""></th1<></th1<></th1<></th1<>
ASOC       S C       12       17       1 - 9"       1 -	$\frac{1}{9'-2''} = \frac{5}{9'-2''} = \frac{5}{3} = \frac{1}{5} = \frac{1}{2'-10''} = \frac{1}{2'-10$
A508 40'-9" 13 STR. A510E 5'-2" 63 2 10" A'-A"	F703 12'-0" 62 STR.
ASTOL     S 2     05     2     10     4 4       ASTOL     2 '-6"     2     10"     1'-8"	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$
A513E       4' - 0"       46       STR.         A514E       5' - 4"       A2       17       2' - 1"       2' - 1"	F705       7'-3"       55       T7       1'-2"       6'-1"       7'/4"
AS14E     S = 4     42     17     2 = 1     1 = 2     2 = 1       AS15E     44' - 9"     7     STR.	W502     26' - 61/2"     35     STR.
A516E 40'-9" 7 STR 7	W503       25' - 9 <sup>1</sup> / <sub>2</sub> "       35       STR.         W506       15' - 3"       28       STR
AST/E     S - S     7     16     2 - 1     1 - 2       AST/E     S - S     7     16     2 - 1     1 - 2       AST/E     S - S     7     16     2 - 1     1 - 2       AST/E     S - S     7     16     2 - 1     1 - 2       AST/E     5' - 10"     20     17     1' - 7"     2' - 8"     1' - 7"	W500     15 - 3     20     51R.       W507     15' - 3½"     27     STR.
A519E         6' - 6"         20         17         1' - 7"         1' - 7"           A500         7' - 2"         10         6TP	W508         7' - 10"         55         17         2' - 1"         1' - 2"         4' - 7"         Image: Comparison of the second s
A520 7'-2" 12 STR. A521 7'-3 <sup>1</sup> //" 12 STR.	W703         15' -9"         28         STR.         Image: Control of the second s
A527E 4'-6" 14 STR.	W704       15' - 10"       27       STR.
A528E 7'-2" 7 STR. A529E 7'-3 <sup>1</sup> //" 7 STR.	
A530E     3' - 11"     7     2     1' - 10"     2' - 1"       A530E     5' - 2"     -     -     -     -	
A531       5' - 8"       13       16       3' - 7"       1' - 9"       1' - 8'/4"       5%"       4' - 0%"         A532E       5' - 8"       14       16       3' - 7"       2' - 1" $2' - 0'/4"$ $6'/4"$ $4' - 1'/4"$	
A533 3'-7" 13 2 1'-10" 1'-9"	
A534E 46'-8" 6 STR. A535 9'-0" 63 STR.	
A536E       6' - 0"       102       STR.	
A537 15'-3" 4 STR. A538E 15'-3" 4 STR.	
A539 15' - 3½" 4 STR.	
A540E 15' - 3 <sup>1</sup> / <sub>2</sub> " 4 STR.	
A542E     4' -9"     56     STR.	Image: series of the series
A704 9'-6" 56 STR	
A705         15' -9"         3         STR.	Image: series of the series
A706 15' -9 <sup>1</sup> / <sub>2</sub> " 4 STR.	
F501 4'-5" 134 STR.	
F502       49' - 4"       4       STR.         F503       40' - 7"       21       STR	
F503     49 -7     21     STR.       F504     36' -7"     6     STR.	
F505         34' -1"         2         STR.         Image: STR.	
F506 $35'-6''$ 10       2       STR. $\Delta = 1'-6''$ $\Delta = 1'-6''$	
F507     27' -8"     8     STR.       F508     22' -0"     2     STR	
F 500         22 -0         2         310         1 <th1< th="">         1         <th1< th=""> <th1< th="">         1         <th1< <="" td=""><td></td></th1<></th1<></th1<></th1<>	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
F510         52 - 10         4         51R.         6         51R.         6         51R.         6         6         51R.         6         6         6         7 <th7< th="">         7         <th7< th=""> <th7< th=""></th7<></th7<></th7<>	
F512     35' -0"     1     STR.       F517     07/ 7"     11     STR.	
F513     25' - 5"     11     STR.       F514     34' - 5"     1     STR.	
F515 25' -8" 11 STR.	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
F602         12' -0"         35         STR.           F603         9' -6"         5         STR.	
F604         6' - 1"         5         STR.	
F605 $\frac{8^{1}/2''}{3'-3^{1}/3''}$ 2       STR.         LENGTH $\wedge = 2'-7''$	
F606     4' - 3"     5     STR.	
F607         10' - 6"         128         STR.           F608         6' - 11"         10         STR	

![](_page_21_Picture_1.jpeg)

		CON
	US 301	T200
	LEVELS ROAD	COL
	TO SUMMIT BRIDGE BOAD	
		NEW

![](_page_21_Figure_5.jpeg)

![](_page_21_Figure_6.jpeg)

![](_page_21_Figure_7.jpeg)

(T7)

				1-479 BR-2
RACT	BRIDGE NO.	1_479		SHEET NO.
911303			SANDY BRANCH	544
NTY	DESIGNED BT.		ABUTMENT 2	TOTAL SHTS.
CASTLE	CHECKED BY:	BCM	REINFORCEMENT SCHEDULE	1256

![](_page_22_Figure_0.jpeg)

![](_page_22_Figure_2.jpeg)

		ADDENDUMS	/ REVISIONS
DEPARTN	DFI AWARE		
	DEDADTMENT OF TRANSPORTATION		
	DEPARTIMENT OF TRAINSPORTATION		

SCALE	US 301	CONTRACT	BRIDGE NO.	1–479
	LEVELS ROAD	COUNTY	DESIGNED BY:	ВМ
FEET	TO SUMMIT BRIDGE ROAD	NEW CASTLE	CHECKED BY:	JW

- 1. ELASTORMERIC BEARINGS SHALL BE 60 DUROMETER HARDNESS SHORE TYPE A.
- 2. MINIMUM LOW TEMPERATURE ELASTOMER SHALL BE GRADE 3.
- 3. ALL BEARINGS ARE TO BE MOLDED TO DESIGN DIMENSIONS. CUTTING TO SIZE AFTER FABRICATION IS PROHIBITED.
- 4. HOLES ARE NOT PERMITTED IN THE ELASTOMERIC BEARINGS.
- 5. VULCANIZE PATCH PIN GROOVES.
- 6. STEEL LAMINATE SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 36 KSI.
- 7. MAXIMUM DESIGN LOAD (SERVICE): FIXED BEARING: DL = 232 KIPS LL = 127 KIPS

EXP.	BEAR ING:	DL	=	232	KIPS
		LL	=	127	KIPS

![](_page_22_Picture_15.jpeg)

![](_page_22_Picture_16.jpeg)

1-479 BD-1 SHEET NO. 545 OTAL SHT 1256

![](_page_23_Figure_0.jpeg)

SCALE		CONTR
0 4 16		T20091
2 8 FEET	TO SUMMIT BRIDGE ROAD	COUN NEW C7

- 1. PRESTRESS CAMBER AND DEAD LOAD DEFLECTION DATA SHOWN IS THEORETICAL AND MAY VARY WITH CONCRETE STRENGTH, VARIABLE PRESTRESSING CONDITIONS AND PRESTRESS LOSSES. CONTRACTOR SHALL VERIFY VALUES IN THE FIELD.
- 2. THE THICKNESS OF THE GIRDER HAUNCHES SHALL BE VARIED TO COMPENSATE FOR ANY INACCURACIES IN THE FINAL CAMBER.

- 1. FOR GENERAL PLAN AND ELEVATION, SEE DWG.
- 2. FOR TYPICAL SECTION, SEE DWG. NO. 1-479 TS-1.
- 3. FOR GIRDER PLAN AND ELEVATION, SEE DWG.
- 4. FOR GIRDER DETAILS, SEE DWG. NO.1-479 BM-2.
- 5. FOR INTERMEDIATE AND END DIAPHRAGM DETAILS, SEE DWG. NO. 1-479 DPH-1.

				1-479 FR-1
TRACT	BRIDGE NO.	1_479		SHEET NO.
911303			SANDY BRANCH	546
UNTY	DESIGNED BY:	LI		TOTAL SHTS.
CASTLE	CHECKED BY:	BCW	FRAMING PLAN	1256

![](_page_24_Figure_0.jpeg)

	CONT
US 301	T2009
LEVELS ROAD	COU
TO SUMMIT BRIDGE ROAD	NEW C

# NOTES:

- 1. NOT ALL PRESTRESSING STRANDS SHOWN FOR CLARITY. CENTER OF GRAVITY FOR STRANDS GROUP (CGS) SHOWN ONLY.
- 2. GIRDER LENGTHS IN CASTING BED SHALL BE DETERMINED AND DEPICTED IN SHOP DRAWINGS TO COMPENSATE FOR GRADE SHORTENING DUE TO PRESTRESS EFFECTS.
- SECTION IS A PCEF 7147. 3.
- ALL MILD STEEL REINFORCEMENT IN GIRDERS SHALL BE EPOXY COATED. 4.
- TOP SURFACE OF GIRDERS SHALL BE ROUGH FINISHED TO A FULL AMPLITUDE OF A 1/4" AND SCRUBBED TRANSVERSELY WITH A COURSE WIRE BRUSH TO REMOVE ALL LAITANCE TO PRODUCE A ROUGHENED SURFACE FOR BONDING.
- 6. END ZONE REINFORCEMENT MAY BE INCREASED BY THE FABRICATOR TO REFLECT FABRICATORS EXPERIENCE AND/OR TO CONTROL CRACKING.
- 7. NO CLEAR COVER LESS THAN AS SHOWN ON THESE PLANS WILL BE ACCEPTED.
- 8. OMIT SLEEVES AND THREADED INSERTS ON THE EXTERIOR FACE OF FASCIA GIRDERS.
- 9. DIMENSION REFERENCED FROM C/L OF GIRDER.
- 10. FABRICATOR TO CHECK GIRDER STABILITY FOR HANDLING, TRANSPORTATION AND ERECTION.

- 1. FOR GENERAL PLAN AND ELEVATION, SEE DWG. NO. 1-479 PE-1.
- 2. FOR GENERAL NOTES, SEE DWG. NO. 1-479 GN-1.
- 3. FOR TYPICAL SECTION, SEE DWG. NO. 1-479 TS-1.
- 4. FOR FRAMING PLAN, SEE DWG. NO. 1-479 FR-1.
- 5. FOR GIRDER DETAILS, SEE DWG. NO. 1-479 BM-2.
- 6. FOR CAMBER DIAGRAM, SEE DWG. NO. 1-479 FR-1.
- 7. FOR INTERMEDIATE AND END DIAPHRAGM DETAILS, SEE DWG. NO. 1-479 DPH-1.

				1-479 BM-1
RACT	BRIDGE NO.	1_479		SHEET NO.
11303			SANDY BRANCH	547
NTY	DESIGNED BI:		GIRDER PLAN	TOTAL SHTS.
ASTLE	CHECKED BY:	BCW	AND ELEVATION	1256
				-

![](_page_25_Figure_0.jpeg)

TRACT	BRIDGE NO.	1_479		SHEET NO.
911303		1-475	SANDY BRANCH	548
UNTY	DESIGNED BY:	LT		TOTAL SHTS.
CASTLE	CHECKED BY:	BCW	GIRDER DETAILS	1256

![](_page_26_Figure_0.jpeg)

	110 001	CONT
		T2009
	LEVELS ROAD	COL
	TO SUMMIT BRIDGE ROAD	NEW (

![](_page_27_Figure_0.jpeg)

					1-479 DK-1
110 004	CONTRACT	BRIDGE NO.	1_479	BAMD E OVER	SHEET NO.
US 301	T200911303		1 470	SANDY BRANCH	550
LEVELS ROAD	COUNTY	DESIGNED BY:	PRH	DECK POURING	TOTAL SHTS.
TO SUMMIT BRIDGE ROAD	NEW CASTLE	CHECKED BY:	JW	SEQUENCE	1256
	US 301 LEVELS ROAD TO SUMMIT BRIDGE ROAD	US 301 LEVELS ROAD TO SUMMIT BRIDGE ROADCONTRACTCOUNTYCOUNTYNEW CASTLE	US 301 LEVELS ROAD TO SUMMIT BRIDGE ROADCONTRACT T200911303BRIDGE NO.COUNTYDESIGNED BY:COUNTYCOUNTYNEW CASTLECHECKED BY:	CONTRACT     BRIDGE NO.     1-479       LEVELS ROAD TO SUMMIT BRIDGE ROAD     COUNTY     DESIGNED BY:     PRH       NEW CASTLE     CHECKED BY:     JW	US 301 LEVELS ROAD TO SUMMIT BRIDGE ROAD           CONTRACT T200911303           BRIDGE NO.           1-479           RAMP F OVER SANDY BRANCH         DESIGNED BY: PRH             COUNTY           DESIGNED BY: PRH           DECK POURING SEQUENCE

![](_page_28_Figure_0.jpeg)

						1-479 DK-2
		CONTRACT	BRIDGE NO.	1_479		SHEET NO.
04		T200911303		1 47 0	SANDY BRANCH	551
2	LEVELS ROAD	COUNTY	COUNTY DESIGNED BY: EE		DECK AND BARRIER	TOTAL SHTS.
FE	TO SUMMIT BRIDGE RO	AD NEW CASTLE	CHECKED BY:	BCW	REINFORCEMENT	1256

- 1. FOR TYPICAL DECK AND PARAPET REINFORCEMENT SECTIONS,
- 2. FOR FINISHED DECK ELEVATIONS, SEE DWG. NO. 1-479 FD-1.
- 3. FOR REINFORCING BAR SCHEDULE, SEE DWG. NO. 1-479 BR-3.
- 4. FOR BARRIER CONTROL JOINT DETAIL, SEE DWG. NO. 1-479 DK-3.

![](_page_29_Figure_0.jpeg)

		110 004	CONT
0 1 SCALE	4		T2009
 1 1 1 2		LEVELS ROAD	COU
FEET		TO SUMMIT BRIDGE ROAD	NEW (

![](_page_30_Figure_0.jpeg)

SCALE	110 201	CONTRACT	BRIDGE NO.	1–479	RAMP F OVER	SHEET N
0 2 8		T200911303	DESIGNED BY: EF		SANDY BRANCH	553
	TO SUMMIT BRIDGE ROAD	TO SUMMIT BRIDGE BOAD			APPROACH SLAB AND	TOTAL SH
FEET		NEW CASILE	CHECKED RI: JM			1256

![](_page_31_Figure_0.jpeg)

NTRACT	BRIDGE NO.	1_479		SHEET N
011303		1 470		554
911202			SANDY BRANCH	554
)UNTY	DESIGNED DI.			TOTAL SH
CASTLE	CHECKED BY:	JW	AFFRUALE SLAD DETAILS	1256

							DECK	BAR S	CHEDL	JLE		
MARK	LENGTH	NO. BARS	TYPE	Α	В	С	D	E	G	Н	J	К
S501E	47' - 7"	281	1	7″	46' -5"	-	_		7"		5"	
S502E	46′ - 7 <sup>1</sup> /2"	281	STR.								-	
S503E	55' -0"	115	STR.									
S504E	53' - 3"	115	STR.									
S505E	36' - 7"	115	STR.	1/ 0//	1/ 0//							
5006E	<u>2'</u> - 4" 5' - 7"	20		1 7	1° - 2*							
S508E	<u> </u>	60	16	2' - 1"	1' - 2 <sup>1</sup> /3"	<u>3' - 81//"</u>	1'-7"			1' - 1 <sup>1</sup> //"		1' - 2"
S509E	46' - 7 <sup>1</sup> / <sub>2</sub> "	4	STR.	2 .	. 272	0 0 72	. ,			/2		. 2
S601E	8' -0"	562	1	8″	7' - 4"						6″	
						APPR	OACH	SI AB	BAR S	CHEDUI	F	
<u>45501</u> ⊑	46' - 7 <sup>1</sup> /"	86	STP								_	
AS502F	29' - 8"	94	STR.									
AS503E	<u> </u>	188	2	3' -0"	1′-0 <sup>1</sup> ⁄⁄″				3' -0"			
AS504E	43' -9"	20	STR.									
AS505E	8′ -7″	94	T1	7″	1' -8"	2' -0 <sup>1</sup> /2"	1′-8″	2' -0 <sup>1</sup> /2"	7″			
AS506E	9′ -1¼″	44	STR.									
AS507E	2′ -6″ T0 8′ -0″	6	STR.									
AS508E	1′-6″ TO 7′-6″	6	STR.									
AS509E	9′ -8″	8	STR.									
AS510E	11' -9 <b>¾</b> "	8	STR.									
AS511E	9'-8" TO 11'-9"	88	STR.									
AS601E	3' -0"	94	STR.									
AS602E	$46' - \frac{1}{2''}$	10	SIR.	0"	0/-11/"						6"	
AS604E	9 -9 74 2' -6" TO	6	STR.	0	9 - 1 /4						0	
AS605E	1' -6" TO	6	STR.									
	/ -0		CTD									
AS701E	46' - / 1/2"	62	SIR.									
AS801F	7' - 3"	176	STR.									
AS802E	4' -0"	16	STR.									
S1001E	29' -8"	182	STR.									
						DIA	PHRA	GM BA	R SCH	EDULE		
	<u>8′_1″</u>	てつ	T1	A1/."	۳.	<u> </u>	۴″	3'-2"	1/."			
M402F	11' - 10"	56	<u>S4</u>	<u>+ /2</u> 9"	4' - 11"	<u> </u>	<u> </u>	5-2	<b>7</b> /2 9"			
M403E	8' -5"	32	T1	 4 <sup>1</sup> ⁄/;"	8"	3' -2"		3' -2"	41/2"			
M404E	7' - 7"	108	2	9"	6' -10"							
M405E	6' -8"	72	2	3' -0"	8″				3' -0"			
M406E	5' -6"	36	2	9"	4' -9"							
	01 0"	E A	СТП									
	0'-9" 2'-5"	04 129	51K. CTD									
M502E	<u> </u>	120	STR.									
M504F	9' - 0"	64	STR.									
M505E	5' - 7"	48	STR.									
M506E	2' -9"	32	STR.									
M507E	6' -11"	12	STR.									
<b>V</b> 508E	5' -9"	12	STR.									
						'				I		

DELAWARE DEPARTMENT OF TRANSPORTATION

ADDENDUMS / REVISIONS

# REINFORCING BAR SCHEDULE

![](_page_32_Figure_5.jpeg)

					1-479 BR-3
	110 004	CONTRACT	BRIDGE NO. <b>1_479</b>		SHEET NO.
	US 301	T200911303		SANDY BRANCH	555
	LEVELS ROAD	COUNTY	DESIGNED BY: LT	DECK REINFORCEMENT	TOTAL SHTS
	TO SUMMIT BRIDGE ROAD	NEW CASTLE	CHECKED BY: BCM	SCHEDULE	1256

J	К	0	R	REMARKS
			3 <sup>3</sup> ⁄4″	
4″	10″		33/4"	
 4″	10"		33/4"	

![](_page_32_Figure_8.jpeg)

![](_page_33_Figure_0.jpeg)

U	ELA	WAKE
DEPARTMENT	OF	TRANSPORTATION

	110 004	CONT
		T2009
		COU
		NEW C

- 1. FOR VERTICAL CURVE DATA, SEE DWG. NO. 1-479 PE-1.
- 2. FOR DECK POURING SEQUENCE, SEE DWG. NO. 1-479 DK-1.
- 3. FOR DECK REINFORCEMENT PLAN, SEE DWG. NO. 1-479 DK-2.
- 4. FOR DECK DETAILS, SEE DWG. NO. 1-479 DK-3.

				1-479 FD-1	
TRACT	BRIDGE NO.	1_479		SHEET NO.	
911303	BRIBGE HOI	1-475	SANDY BRANCH	556	
JNTY	DESIGNED BY:	LT		TOTAL SHTS.	
CASTLE	CHECKED BY:	JW	ELEVATIONS	1256	

![](_page_34_Figure_0.jpeg)

	CONT
	T2009
LEVELS ROAD	COU
TO SUMMIT BRIDGE ROAD	NEW C

- FOR APPROACH SLAB REINFORCEMENT PLAN, SEE DWG. NO. 1-479 AS-1.
- 2. FOR APPROACH SLAB DETAILS, SEE DWG. NO. 1-479 AS-2.
- 3. FOR VERTICAL CURVE DATA, SEE DWG. NO. 1-479 PE-1.

				1-479 FD-2
RACT	BRIDGE NO.	1_479		SHEET NO.
911303	;	1 470	SANDY BRANCH	557
NTY	DESIGNED BY:	PRH	FINISHED APPROACH	TOTAL SHTS.
CASTLE	CHECKED BY:	JW	SLAB ELEVATIONS	1256

![](_page_35_Figure_0.jpeg)

![](_page_35_Figure_3.jpeg)

NT OPENING (INCH)							
TEMPERATURE (°F)							
30	40	50	60	70	80		
2 <sup>1</sup> /8	21/16	21/16	2	2	1 <sup>1</sup> 5%6		
2 %	2 <mark>3</mark> /8	2 <sup>1</sup> /4	2 <sup>1</sup> /8	2	1 <sup>13</sup> /16"		

5. CONSTRUCT EXPANSION JOINT TO MATCH ROADWAY GRADE AND CROSS SLOPE.

6. GRIND ALL STEEL EDGES EXPOSED TO TRAFFIC TO 3/16" MIN. RADIUS.

BOND NEOPRENE STRIP SEAL TO EXTRUSION WITH APPROVED ADHESIVE.

				1-479 EX-1
RACT	BRIDGE NO.	1_479		SHEET NO.
911303		1-475	SANDY BRANCH	558
INTY	DESIGNED BY:	PRH		TOTAL SHTS.
CASTLE	CHECKED BY:	JW	EXPANSION JUINT DETAILS	1256

![](_page_36_Figure_0.jpeg)

DELAWARE **DEPARTMENT OF TRANSPORTATION** 

DHEAD	BOLT
ΡΙΡΕ	CAP)

# - 4" HIGH OAKUM OR EQUIVALENT COMPRESSIBLE MATERIAL TO BE PLACED BETWEEN SETTLEMENT PLATE AND BOTTOM OF CASING

<u>NOTES:</u>

1. THE BASE OF THE SETTLEMENT PLATFORM SHALL BE PLACED ON THE TOP OF THE EXISTING GROUND.

2. READINGS ON THE SETTLEMENT PLATFORMS SHALL BE MADE AFTER THE INITIAL INSTALLATION OF THE RISER AND CASING PIPES AND INSTALLATION RECORD SHEETS ARE APPROVED BY THE ENGINEER AND PRIOR TO FILL PLACEMENT. DURING FILL PLACEMENT, READINGS ON ALL SETTLEMENT PLATFORMS SHALL BE TAKEN AT A MINIMUM OF THREE (3) CALENDAR DAY INTERVALS. AFTER COMPLETION OF THE FILL AND SURCHARGE PLACEMENT, INSTALL SETTLEMENT MONUMENTS IF INDICATED ON THE BRIDGE PLANS AND TAKE INITIAL READINGS. READINGS ON ALL SETTLEMENT MONITORING DEVICES SHALL THEN BE TAKEN AT A MINIMUM OF THREE (3) CALENDAR DAY INTERVALS. AFTER THE FILL HAS BEEN COMPLETED AND TWO (2) SUCCESSIVE READINGS OF EACH DEVICE HAS RECORDED LESS THAN OR EQUAL TO 0.1 INCH, THE IMMEDIATE SETTLEMENT WILL BE DEEMED COMPLETE AND THE GEOTECHNICAL ENGINEER CAN RELEASE THE SUBSTRUCTURE FOR REMOVAL OF SURCHARGE AND INSTALLATION OF PRODUCTION PILES. AFTER COMPLETION OF THE ABUTMENT, THE CONTRACTOR SHALL ESTABLISH REFERENCE POINTS TO MONITOR SETTLEMENT ON TOP OF THE ABUTMENT SEAT WITHIN 5 FEET OF ALL ENDS AND CORNERS AND AT THE CENTER OF BRIDGES AND THE CENTERLINE OF US301. AFTER THE CONCRETE ABUTMENTS HAVE BEEN CONSTRUCTED, READINGS ON ALL SETTLEMENT MONITORING DEVICES AND REFERENCE POINTS SHALL CONTINUE TO BE TAKEN AT A MINIMUM OF THIRTY (30) DAY INTERVALS FOR THE NEXT SIX (6) MONTHS OR AS DIRECTED BY THE ENGINEER.

	110 004	CONT
		T2009
		COL
	TO SUMIMIT BRIDGE ROAD	NEW (

SETTLEMENT PLATFORM	STATION	OFFSET	SETTLEMENT MONUMENT	STATION	OFFSET
SP-1-479-1	45+00.85	14.25′RT	SM-1-479-1	45+02.19	9.25′RT
SP-1-479-2	46+70.85	14.25′RT	SM-1-479-2	46+72.19	9.25′RT

![](_page_36_Picture_16.jpeg)

CROSS REFERENCE NOTE:

FOR LOCATION OF SETTLEMENT PLATFORMS AND MONUMENTS, SEE DWG. 1-479 PE-1.

1-479 DT-1

RACT	BRIDGE NO.	1_479		SHEET NO.
911303				559
	DESIGNED BY: PRH		SANUT DRANCH	
INTY			SETTLEMENT PLATFORM	TOTAL SHTS.
CASTLE	CHECKED BY:	JW	DETAIL	1256

![](_page_37_Figure_0.jpeg)

	ADDENDUMS	/ REVISIONS			CON
DELAWARE			SCALE US	US 301	T200
DEPARTMENT OF TRANSPORTATION			0 15 30 45	LEVELS ROAD	COI
			FEET	TO SUMIMIT BRIDGE ROAD	NEW